Community Services Department Planning and Building VARIANCE APPLICATION



Community Services Department Planning and Building 1001 E. Ninth St., Bldg. A Reno, NV 89512-2845

Telephone: 775.328.6100

Washoe County Development Application

Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Name: Project Address: Project Address: Project Area (acres or square feet): Project Location (with point of reference to major cross streets AND area locator): Assessor's Parcel No.(s): Parcel Acreage: Address: Address: Zip: Contact Person: Contact Pe	Project Information s		Staff Assigned Case No.:	
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Property Owner Affidavit

Applicant Name:

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

STATE OF NEVADA

COUNTY OF WASHOE

OUS

(please print name)

being duly sworn, depose and say that I am the owner* of the property or properties involved in this application as listed below and that the foregoing statements and answers herein contained and the information herewith submitted are in all respects complete, true, and correct to the best of my knowledge and belief. I understand that no assurance or guarantee can be given by members of Planning and Building.

(A separate Affidavit must be provided by each property owner named in the title report.)

00 Assessor Parcel Number(s) Printed Name Signee 900 Address Subscribed and sworn to before me this (Notary Stamp) day of 2022 COULTON S. MAC GORDEN-STEEDMAN Notary Public - State of Nevada County of Washoe APPT. NO. 21-0220-02 My App. Expires May 24, 2025 Notary Public - State of Nevada Notary Public in and for said county and state My commission expires:

*Owner refers to the following: (Please mark appropriate box.)

X Owner

- Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- Power of Attorney (Provide copy of Power of Attorney.)
- Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- Property Agent (Provide copy of record document indicating authority to sign.)
- Letter from Government Agency with Stewardship

Property Owner Affidavit

CHAE! ERNOCH BARRY Applicant Name:

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

STATE OF NEVADA

COUNTY OF WASHOE

I. BARRY MICHAEL CERNOCH (please print name)

being duly sworn, depose and say that I am the owner* of the property or properties involved in this application as listed below and that the foregoing statements and answers herein contained and the information herewith submitted are in all respects complete, true, and correct to the best of my knowledge and belief. I understand that no assurance or guarantee can be given by members of Planning and Building.

(A separate Affidavit must be provided by each property owner named in the title report.)

Assessor Parcel Number(s): Printed Name BARRY Signed Sch Address 309 HUF Subscribed and sworn, to before me this day of 1027 (Notary Stamp) ATTREETIMETER DESCRIPTION OF THE COULTON S. MAC GORDEN-STEEDMAN Notary Public - State of Nevada County of Washoe Notary Public in and for said county and state APPT, NO. 21-0220-02 My App, Expires May 24, 2025 My commission expires: ANTER CONTRACTOR OF ANTER CONTRACTOR CONTRAC

*Owner refers to the following: (Please mark appropriate box.)

🖄 Owner

- Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- Dever of Attorney (Provide copy of Power of Attorney.)
- Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- Property Agent (Provide copy of record document indicating authority to sign.)
- Letter from Government Agency with Stewardship

Variance Application Supplemental Information

(All required information may be separately attached)

1. What provisions of the Development Code (e.g. front yard setback, height, etc.) must be waived or varied to permit your request?

The side yard setback must be varied to permit our request. The barn was built in 1991 when the side yard setback was 12'. The current setback requirement is 50', but there are only 43' from the property line to the barn to be converted into an ADU.

You must answer the following questions in detail. Failure to provide complete and accurate information will result in denial of the application.

2. What are the topographic conditions, extraordinary or exceptional circumstances, shape of the property or location of surroundings that are unique to your property and, therefore, prevent you from complying with the Development Code requirements?

Not applicable.

3. What steps will be taken to prevent substantial negative impacts (e.g. blocking views, reducing privacy, decreasing pedestrian or traffic safety, etc.) to other properties or uses in the area?

The structure is existing, but does not block any views from neighboring properties nor does it impact pedestrian/traffic safety.

4. How will this variance enhance the scenic or environmental character of the neighborhood (e.g. eliminate encroachment onto slopes or wetlands, provide enclosed parking, eliminate clutter in view of neighbors, etc.)?

The conversion will increase the visual appeal of the existing structure through the renovation of the exterior and addition of window openings to the sides of the building.

5. What enjoyment or use of your property would be denied to you that is common to other properties in your neighborhood?

Due to the acreage of the surrounding properties, many of the neighbors have additional dwelling units on their properties.

6. Are there any restrictive covenants, recorded conditions or deed restrictions (CC&Rs) that apply to the area subject to the variance request?

		Yes	🔳 No	If yes, please attach a copy.
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7. How is your current water provided?

Well (well meter shall be installed and usage shall be approved by the Nevada State Water Engineer)

8. How is your current sewer provided?

Current Residece will continue to use the existing septic, and the proposed ADU will have a new septic system.

ABBREVIATIONS ADD'L ADDITIONAL ALT ALTERNATE A.B. ANCHOR BOLT APPROX APPROXIMATE BM BEAM BRG BEARING BEL BELOW BET BETWEEN BLK BLOCK B/S BOTH SIDES BOT BOTTOM B.N. BOUNDARY NAILING BLDG BUILDING CANT CANTILEVER C.B. CARRIAGE BOLT CLG CEILING CL CENTERLINE CHNL CHANNEL CLR CLEAR COL COLUMN CP COMPLETE PENETRATION CONC. CONCRETE CMU CONCRETE MASONRY UNIT CONT CONTINUOUS CJ CONTROL JOINT C.M.J. CONTROL MASONRY JOINT C/S COUNTERSINK D.L. DEAD LOAD DET DETAIL DIA. DIAMETER DIM DIMENSION DO DITTO DJ DOWEL JOINT DBL DOUBLE DF DOUGLAS FIR DWG DRAWING EA EACH EE EACH END EF EACH FACE ES EACH SIDE EW EACH WAY E.N. EDGE NAIL ELEV ELEVATION EMBED EMBEDMENT EQ EQUAL (E) EXISTING EXP EXPANSION E.B. EXPANSION BOLT EJ EXPANSION JOINT EXT EXTERIOR F.O.C. FACE OF CONCRETE F.O.M. FACE OF MASONRY F.O.S. FACE OF STUD F.N. FIELD NAIL/FACE NAIL FIN FLR FINISH FLOOR FTG FOOTING FEF FORCED-ENTRY FASTNERS FDN FOUNDATION GA GAGE GALV GALVANIZED G.L. GLU-LAM G.L.B. GLUED-LAMINATED BEAM GYP BD GYPSUM BOARD HGR HANGER HSA HEADED STUD ANCHOR HDR HEADER HT HEIGHT HF HEM-FIR HSB HIGH-STRENGTH BOLT HORIZ HORIZONTAL INFO INFORMATION ID NSIDE DIAMETER INT INTERIOR IF ISOLATION JST JOINT KD JOIST KILN DRIED KING KING STUD LVL LAMINATED VENEER LUMBER LT LIGHT LL LIVE LOAD LG LONG LLH LONG LEG HORIZONTAL LLV LONG LEG VERTICAL MB MACHINE BOLT MIW MALLEABLE IRON WASHER MANU'E MANUFACTURER MAX MAXIMUM MECH MECHANICAL ML MICRO-LAM (BY TRUS JST) MIN MINIMUM MISC MISCELLANEOUS (N) NEW N.I.C. NOT IN CONTRACT NTS NOT TO SCALE # NUMBER/POUNDS O.C. ON CENTER O/S ONE SIDE OPP OPPOSITE OH OPPOSITE HAND O.S.B. ORIENTED STRAND BOARD OD OUTSIDE DIAMETER o/ OVFR PSL PARALLAM (BY TRUS JST) PARL or // PARALLEL PP PARTIAL PENETRATION PEN PENETRATION PL PLATE PLY PLYWOOD PSF POUNDS PER SQUARE FOOT PSI POUNDS PER SQUARE INCH P.A.F. POWER ACTUATED FASTENER PDF POWER DRIVEN FASTENER PT PRESSURE TREATED PRT PRESERVATIVE TREATED PL PROPERTY LINE or PLATE R. RADIUS RWD REDWOOD REF REFERENCE REQ'D REQUIRED RMT. ROSBORO MFG. TIMBER SCHED SCHEDULE SAD SEE ARCHITECTURAL DWGS SMD SEE MECHANICAL DWGS STS SELF-TAPPING SCREW SW SHEAR WALL SIM SIMILAR SJ SLAB JOINT S.O.G. SLAB ON GRADE SB SOLID BLOCK SPEC SPECIFICATION SQ SQUARE STD STANDARD STL STEEL SYM SYMMETRICAL THRD THREADED T N TOF NAII T&G TONGUE & GROOVE T&B TOP & BOTTOM T.O. TOP OF TS TUBE STEEL TRMR. TRIMMER TYP TYPICAL UBC UNIFORM BUILDING CODE UNO UNLESS NOTED OTHERWISE VERT VERTICAL WT WEIGHT WS WELDED STUD/WOOD SCREW WWF WELDED WIRE FABRIC WWM WELDED WIRE MESH

ENGINEER OF RECORD:



Dunagan Engineering, Inc. 4790 Caughlin Parkway #766, Reno, NV 89519 P. 775.329.2733 F. 888.873.0790 W. DElengineers.com



SYMBOLS

 ROOM TAG Area Base Finish Ceiling Finish Comment -- ELEV. REF.

View Name 1/8" = 1'-0"

- -

- -- -

71

DOOR

TYPE

NUMBER SHEET NUMBER 🗕 A101

DOOR TYPE

WINDOW TYPE → WINDOW TYPE

ROOM NAME

ROOM INFORMATION -

ELEVATION HEIGHT

DRAWING NAME

DRAWING SCALE

GRID LETTER/NUMBER

SHEET REFERENCE

DETAIL REFERENCE

ELEVATION REFERENCE

SHEET NUMBER

SHEET NUMBER

NUMBER

NUMBER

ELEVATION REFERENCE

ROOM DESIGNATION

DATUM / ELEVATION

DRAWING TITLE

GRID BUBBLE

SECTION DESIGNATION

DETAIL DESIGNATION

EXTERIOR ELEVATION DESIGNATION

DOOR DESIGNATION

WINDOW DESIGNATION

NORTH ARROW

BARN CONVERSION



WIND DESIGN DATA

Ultimate Design Wind Speed, Vu = 120 m.p.h. (3-Second Gust) Risk Category II Wind Importance Factor, Iw = 1.00

Wind Exposure C

Internal Pressure Coefficient = +/- 0.18 Components & Cladding Design Pressures (ASCE 7 Section 30.4.2): a = 3.2 ft (ASCE 7 Figure 30.4-1)

Refer to ASCE 7-16 Figure 30.4-1 for layout.				
Roof/Wall	Zone	Effective Wind Area (ft ²)	Design Wind Pressure, P _{net} (psf)	
	1	10	58.0	
	1	20	58.0	
I	1	50	35.2	
Å	1	100	18.0	
Roof > 20 to 27°	2	10	84.5	
0	2	20	73.1	
~	2	50	58.0	
ğ	2	100	46.5	
Ro	3	10	100.4	
	3	20	86.0	
	3	50	67.0	
	3	100	52.6	
	4	10	34.0	
I	4	20	32.5	
Wall	4	50	30.7	
	4	100	29.3	
	5	10	42.0	
	5	20	39.2	
	5	50	35.5	
	5	100	32.5	

SEISMIC DESIGN DATA

Total Load =

Importance Factor, le = 1.00 (Risk Category II) Ss = 2.212 g and S1 = 0.781 g Site class: = D SDs = 1.474 g , SD1 = 0.885 g Seismic design category: = D Basic seismic-force-resisting system(s): = Light-Framed Walls Sheathed with Wood Structural Panels Rated for Shear Resistance, R = 6.5 N/S Design Base Shear (LRFD) = 9.1 kips E/W Design Base Shear (LRFD) = 9.1 kips Cs (LRFD)= 0.2269 Analysis Procedure Used = Equivalent Lateral Force Procedure SNOW LOAD DATA: 5048 FT. Site Elevation Pg = 31 psf Pf = 21 psf Ground Snow Load Flat-Roof Snow Load Ce = 0.9 Snow Exposure Factor

Snow Importance Factor ls = 1.0 Thermal Factor Ct = 1.1 FLOOR FRAMING DESIGN LOADS 40 PSF Floor Live Load = <u>S.O.G.</u> 40 PSF Floor Dead Load = Total Floor Load = TYPICAL 21 PSF **ROOF LOADING:** Snow Load = 15 PSF 36 PSF Dead Load =

PROPERTY OWNER:

PROPERTY LOCATION:

PROPERTY INFORMATION:

ZONING:: OCCUPANCY GROUP: FIRE SPRINKLERS:

NUMBER OF STORIES: CODE EDITIONS:



PROPERTY OWNER / GENERAL CONTRACTOR :

BARRY CERNOCH 3095 LAKESHORE DR. WASHOE COUNTY, NV 89704 APN:050-340-06

PROJECT DATA

BARRY CERNOCH 4205 SLIDE MOUNTAIN DR. RENO, NV 89511 3095 LAKESHORE DR. WASHOE VALLEY, NV 89704 LAKESHORE FARMS SUBDIVISION LOT 12 9.657 ACRES APN: 530-340-06 LDR R-3 (House) S-2 (Garage)

NONE - NOT REQUIRED PER 2018 INTERNATIONAL RESIDENTIAL CODE

1

2018 INTERNATIONAL RESIDENTIAL CODE (IRC) 2018 INTERNATIONAL BUILDING CODE (IBC) 2018 UNIFORM MECHANICAL CODE 2018 UNIFORM PLUMBING CODE 2017 NATIONAL ELECTRICAL CODE 2018 INTERNATIONAL FIRE CODE ANSI 2017 2018 IECC

2018 NORTHERN NEVADA AMENDMENTS

IR1 N.C. w/ NON-CONFORMING WATER SUPPLY & WITH 30FT DEFENSIBLE SPACE

SUBMITTAL SET

SHEET INDEX

A1.1 EXISTING FLOOR PLAN w/ DEMOLITION

A1.2 EXISTING ELEVATIONS w/ DEMOLITION

SECTIONS / ELECTRICAL FLOOR PLAN

GENERAL NOTES & TYPICAL DETAILS

PROPOSED FLOOR PLAN

PROPOSED ROOF PLAN

A1.5 PROPOSED ELEVATIONS

S0.2 TYPICAL DETAILS

S0.3 TYPICAL DETAILS

S1.1 FOUNDATION PLAN

S1.2 STRUCTURAL FLOOR PLAN

S2.1 ROOF FRAMING PLAN / CEILING JOIST

S0.4 DETAILS

A0.0 COVER SHEET

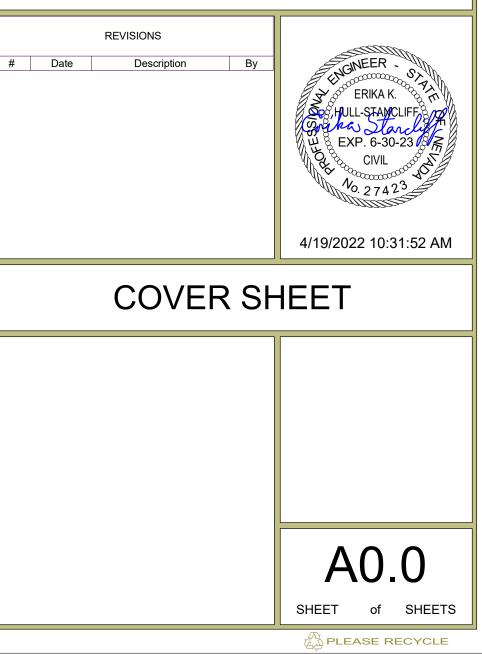
A0.1 SITE PLAN

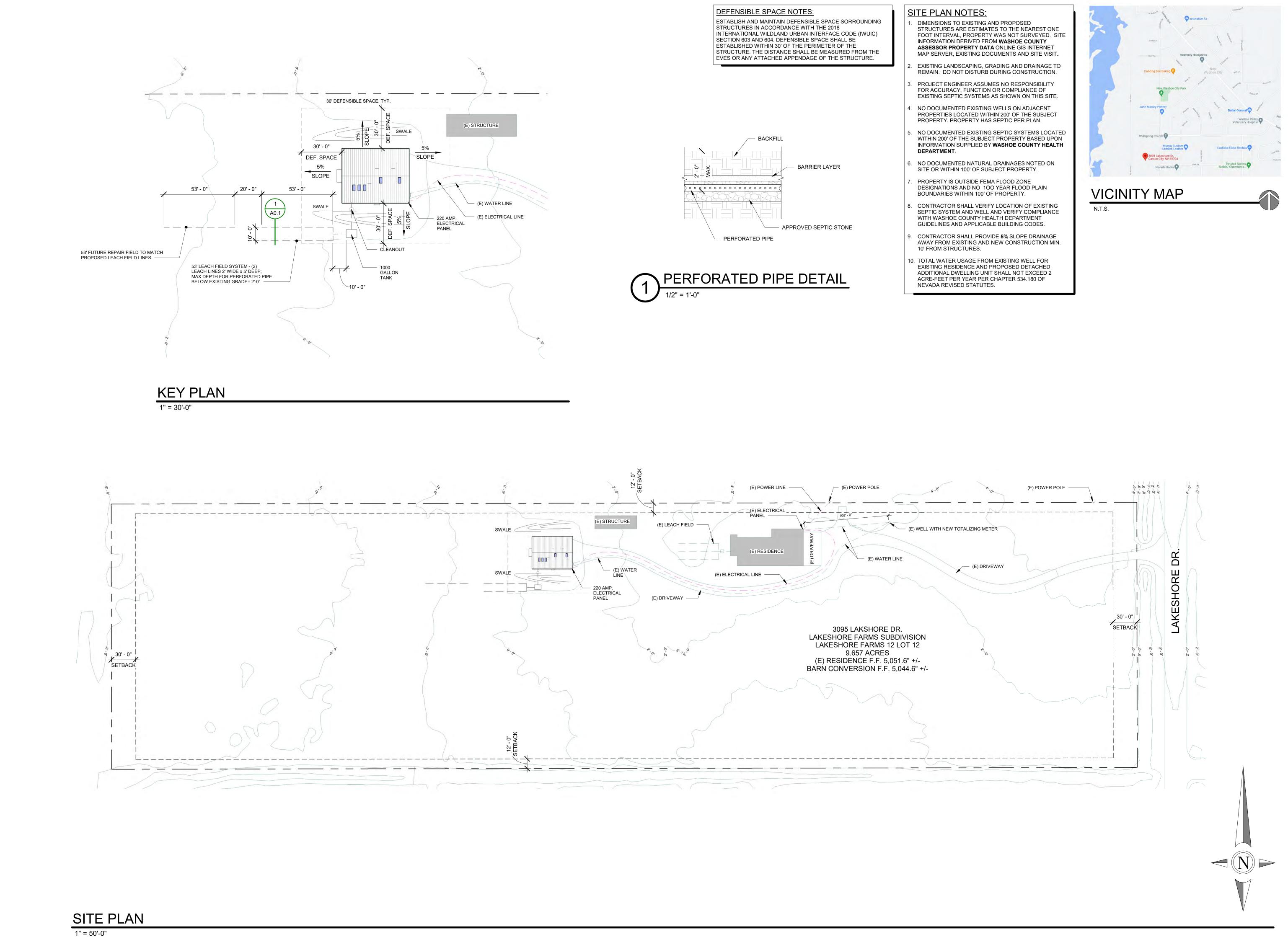
A1.3

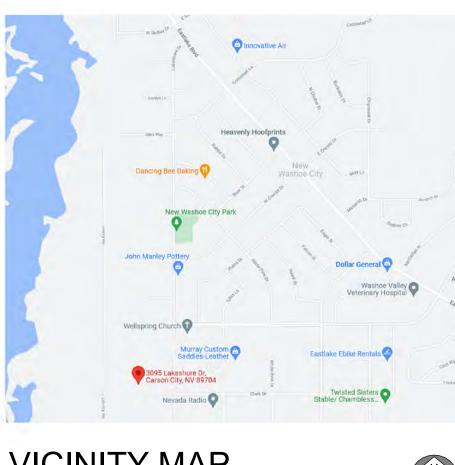
A1.4

A1.6

S0.1

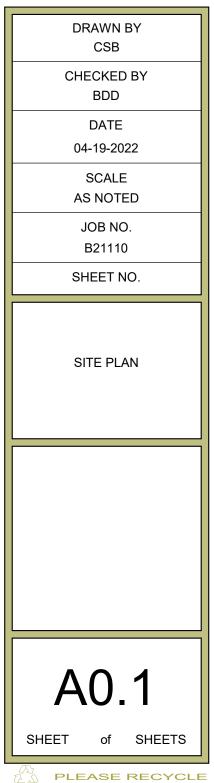


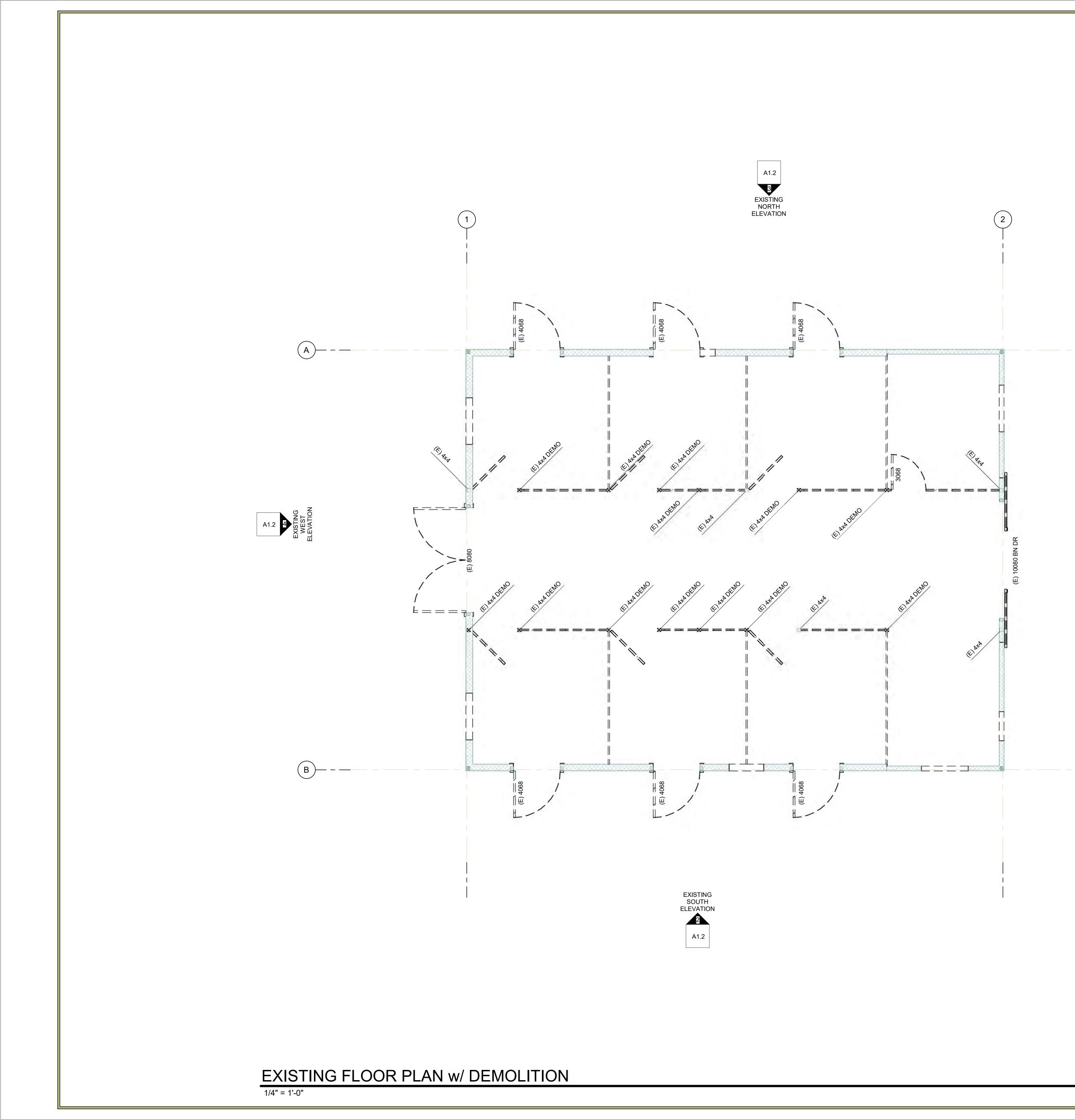




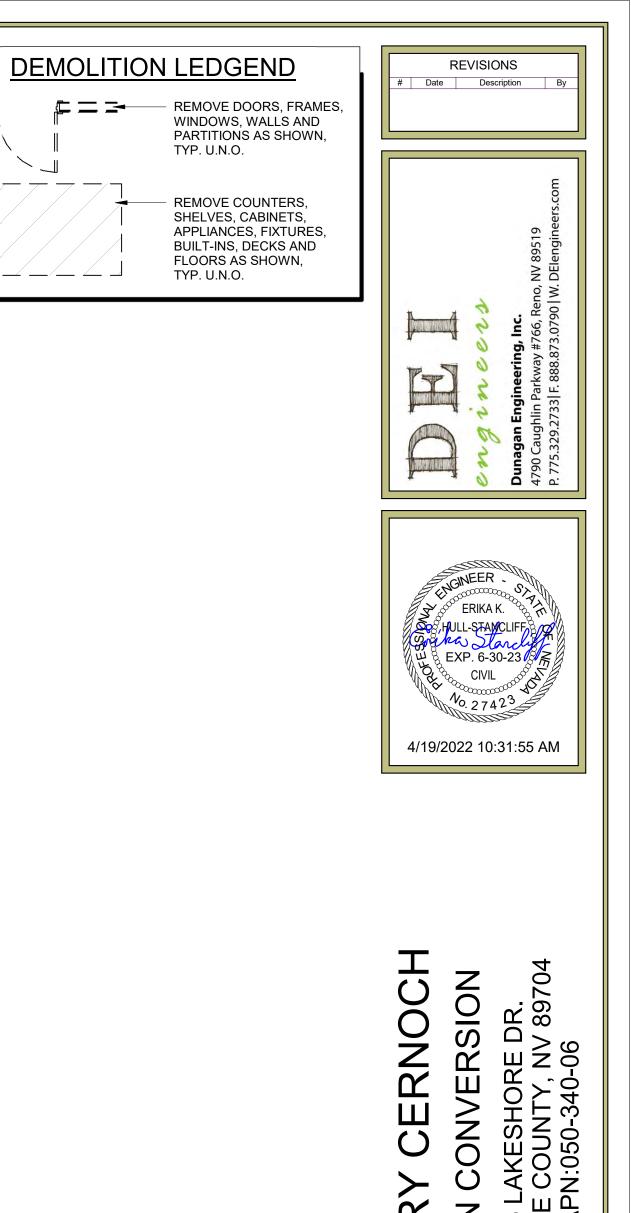


LAKESHORE DR. E COUNTY, NV 89704 PN:050-340-06 CERNOCH NVERSION Ο **BARR'** 3095 L SHOE BARN AP









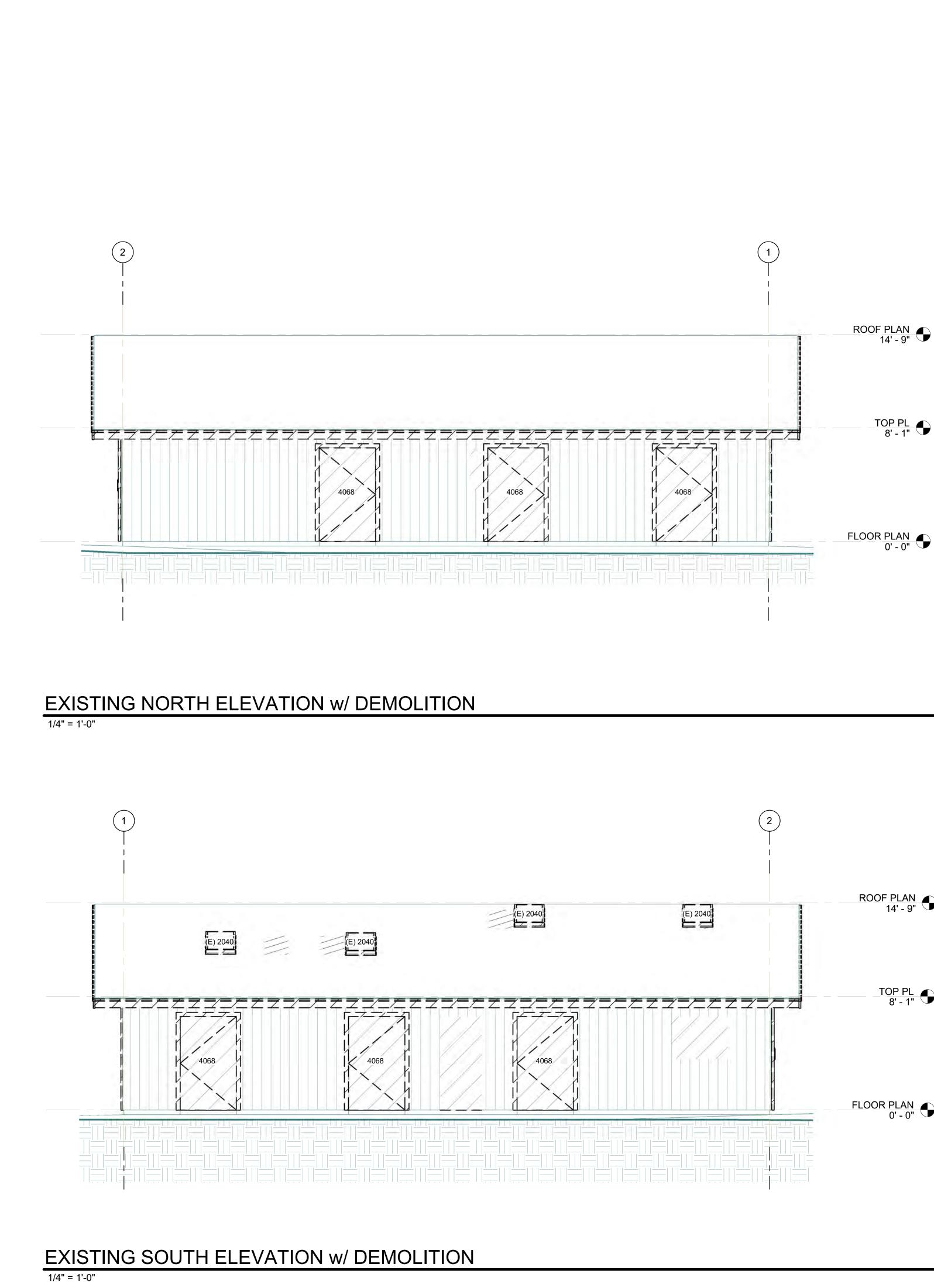
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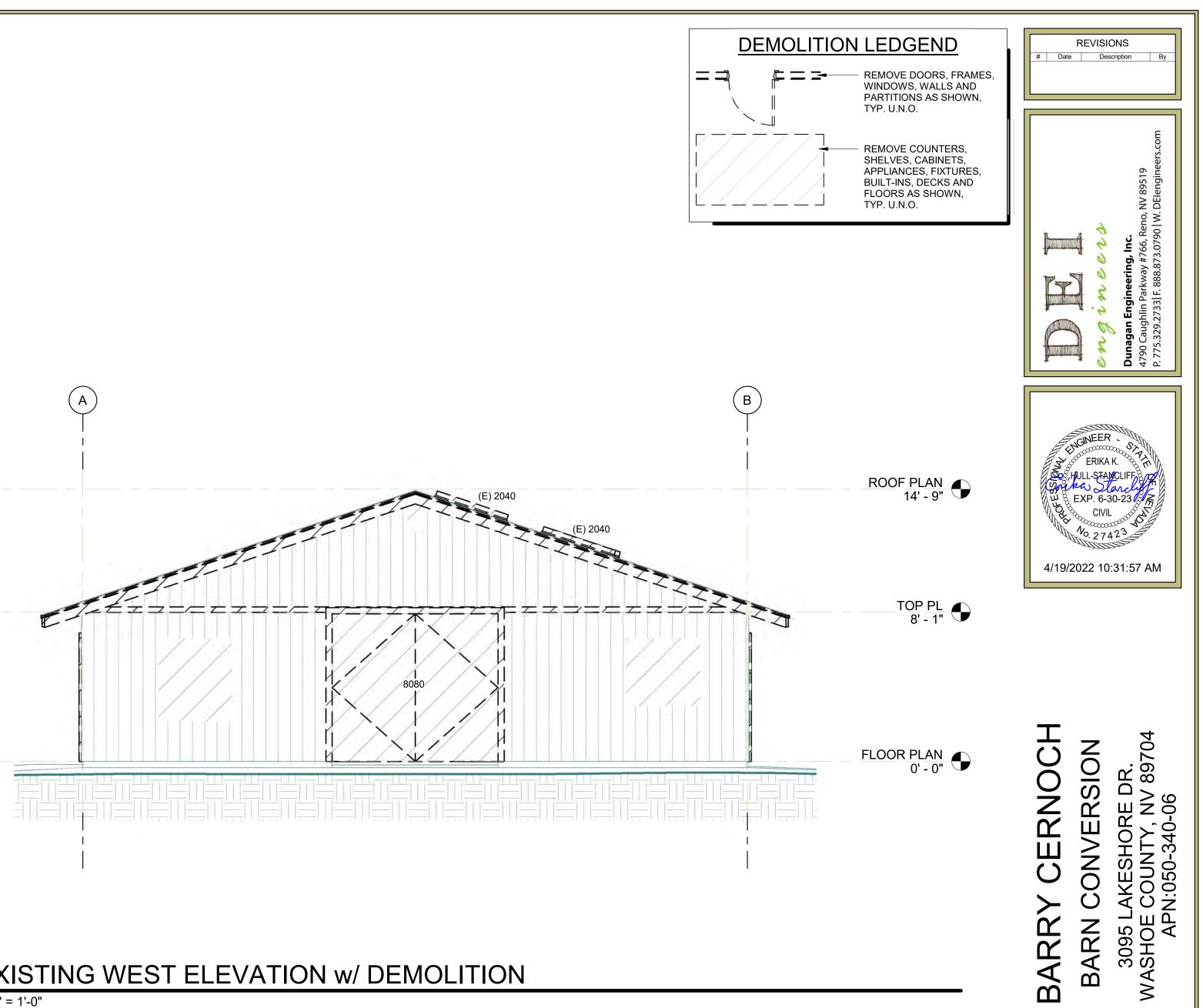
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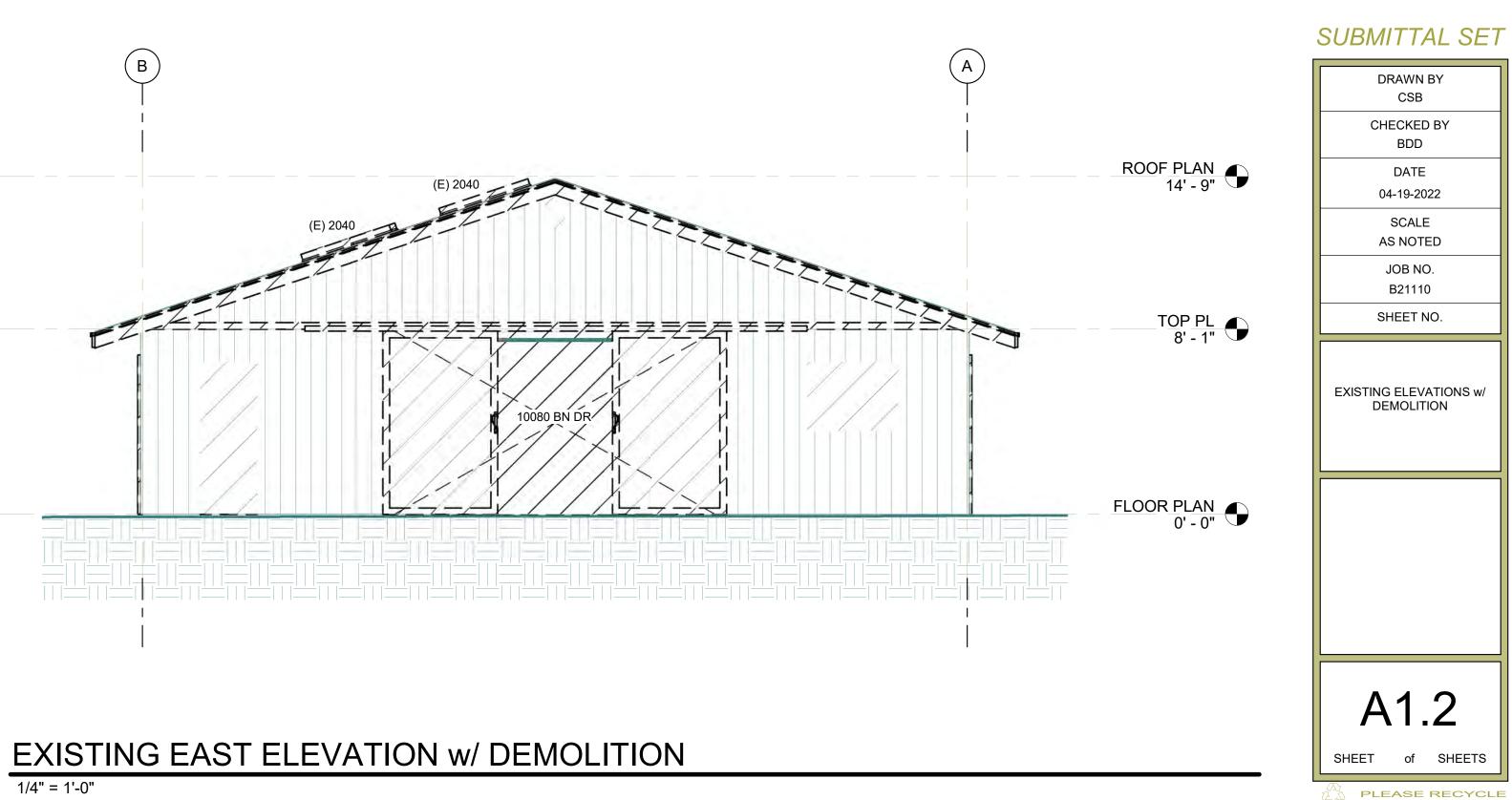
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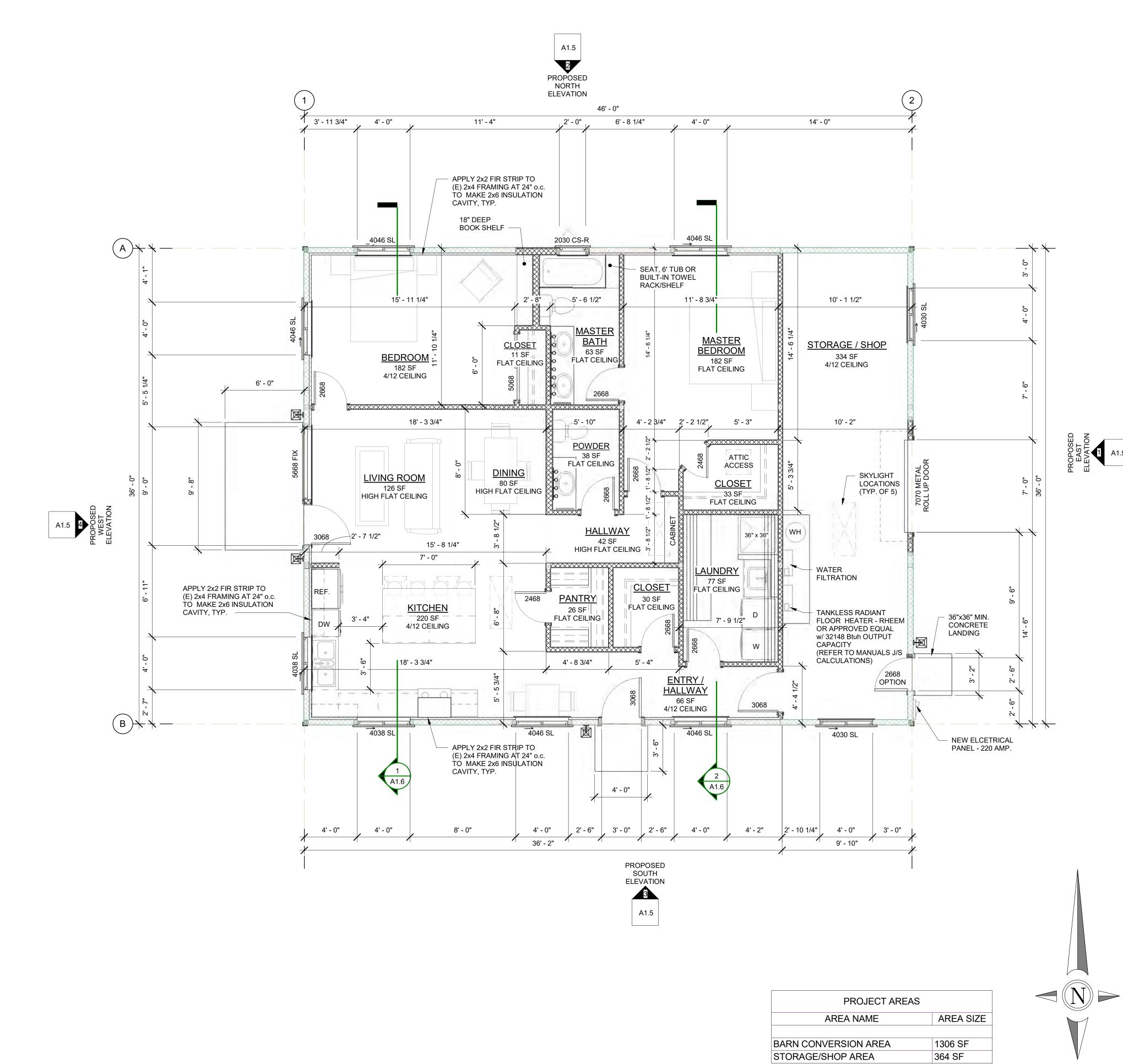




ROOF PLAN 14' - 9" TOP PL 8' - 1" FLOOR PLAN 0' - 0"

EXISTING WEST ELEVATION w/ DEMOLITION 1/4" = 1'-0"





PROPOSED FLOOR PLAN

1/4" = 1'-0"

TYPICAL FLOOR PLAN NOTES:

- 1. GYP. BOARD CEILINGS: 5/8" GYP. BD. CEILINGS TO HAVE FRAMING MEMBERS AT 24" o.c., 1/2" GYP. BD.. CEILINGS TO HAVE FRAMING MEMBERS AT 16" o.c.
- 2. PROVIDE 1/2" GYP. BD. CONTINUOUS ON GARAGE FACE OF HOUSE/GARAGE COMMON WALLS. PROVIDE 1/2" GYP. BD. ON GARAGE CEILING AT HOUSE / GARAGE AS REQUIRED BY LOCAL GOVERNING CODES.
- 3. SLOPE GARAGE FLOOR DOWNWARD 2" TO GARAGE DOOR.
- 4. WATER HEATER: PROVIDE ELEVATED PLATFORM (18" A.F.F.) AND SEISMIC ANCHORAGE PER 2018 IRC, PROVIDE TEMPERATURE AND PRESSURE RELIEF VALVE w/ DRAIN TO EXTERIOR. SUPPLY WATER PRESSURE THROUGH BUILDING. SUPPLY NOT TO EXCEED PRESSURE RELIEF RATING. PROVIDE COMBUSTION AIR. HOT WATER LINES TO HAVE CIRCULATION PUMP OR HOT WATER LINES TO BE MAX. 1/2" DIAMETER. INSULATE ALL HOT WATER LINES TO w/ MIN. R-2 INSULATION.
- 5. EXTERIOR HOSE BIBS TO BE FROST FREE WITH NON-REMOVABLE BACKFLOW PREVENTION DEVICES.
- 6. EMERGENCY EGRESS IN SLEEPING ROOMS SHALL COMPLY WITH GOVERNING FIRE AND BUILDING CODES, MAXIMUM SILL HEIGHT AT EGRESS WINDOW SHALL NOT EXCEED 44" A.F.F. CLEAR OPENING OF 24" HIGH MIN. X 20" WIDE MIN.
- 7. SHOWER AND TUB/SHOWER COMBINATIONS SHALL HAVE A SMOOTH HARD, NON-ABSORBENT SURFACE OVER MOISTURE RESISTANT GYP. BD. TO A HEIGHT OF 70" MIN. DRAIN INLET.
- 8. ALL DIMENSIONS ARE FROM FACE OF STUD TO FACE OF STUD U.N.O.

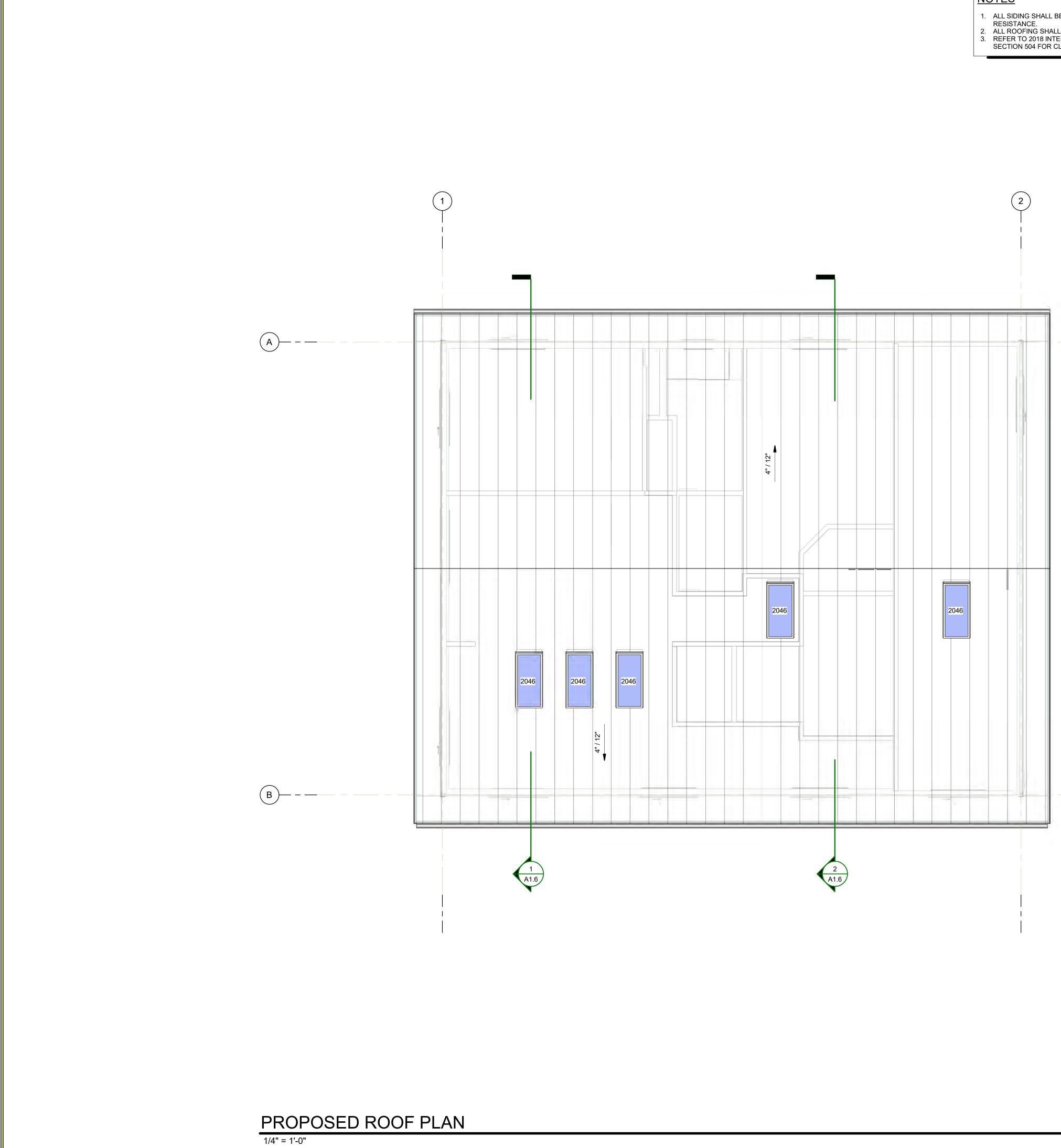
9. ALL EXTERIOR WALLS AND INTERIOR PLUMBING WALLS TO BE INSULATED.

- 10. ALL EXTERIOR DOORS SHALL HAVE A LANDING A MIN. 36" IN THE DIRECTION OF TRAVEL BY WIDTH OF THE DOOR. EXTERIOR DOORS SHALL BE APPROVED NONCOMBUSTIBLE CONSTRUCTION WITH SOLID CORE NOT LESS THAN 1 3/4" THICK OR HAVE FIRE PROTECTION RATING OF NOT LESS THAN 20 MIN. COMPLY WITH REQUIREMENTS OF THE 2018 IWUIC SECTION 504.9.
- 11. PROVIDE FIRE-BLOCKING AT 10' MAX.
- 12. ALL APPLIANCES, MECHANICAL UNITS, PLUMBING FIXTURES, LIGHTING FIXTURES, FIREPLACE, ETC. WITH BRAND, MODEL NUMBER AND SIZE TO BE SUPPLIED TO CONTRACTOR, BY OWNER, PRIOR TO CONSTRUCTION.
- 13. MIN. CLEARANCE FOR STUCCO WEEP SCREED TO BE 4" MIN. AT EARTH AND 2" MIN. AT CONC./ASPHALT/PAVERS. PROVIDE WATER RESISTANT BARRIER PER R703.2/R703.63
- 14. PRESSURE REDUCING VALVES REQUIRED ON INCOMING WATER SERVICE.
- 15. WHERE WATER HEATER VENTS PASS THROUGH INSULATION ASSEMBLIES AND INSULATION SHIELD CONSTRUCTED OF NOT LESS THAN 26 GA. SHEET METAL AND EXTENDING 2" ABOVE INSULATION SHALL BE INSTALLED AS PER 2018 IRC SECT. G2426.4.
- 16. DESIGNATE SAFETY GLAZING PER IRC R308.
- 17. WINDOW & SKYLIGHTS U-FACTOR 0.30 MIN. ALL WINDOW GLAZING SHALL MEET THE REQUIREMENTS OF SECTION 504.8 OF THE 2018 IWUIC OR HAVE FIRE RATING OF 20 MIN.
- 18. FIRESTOP ANY ROOF PROFILE WITH SPACE BETWEEN ROOF DECKING AND ROOF COVERING PER SECTION 504.2 OF THE 2018 IWUIC.





DRAWN BY
CSB
CHECKED BY
BDD
DATE
04-19-2022
SCALE
AS NOTED
JOB NO.
B21110
SHEET NO.
PROPOSED FLOOR PLAN
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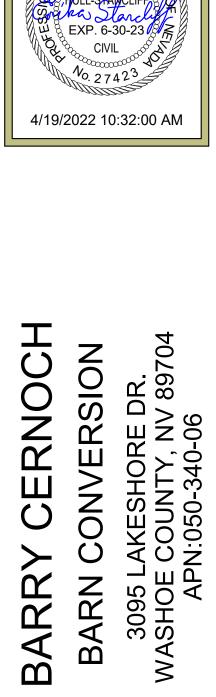


<u>NOTES</u>

- ALL SIDING SHALL BE NON-COMBUSTIBLE, 1-HOUR MIN. FIRE RATED, OR IGNITION RESISTANCE.
 ALL ROOFING SHALL BE CLASS A OR APPROVED NONCOMBUSTIBLE MATERIAL.
 REFER TO 2018 INTERNATIONAL WILDLAND URBAN INTERFACE CODE (IWUIC) SECTION 504 FOR CLASS 1 IGNITION-RESISTANCE CONSTRUCTION REQUIREMENTS.

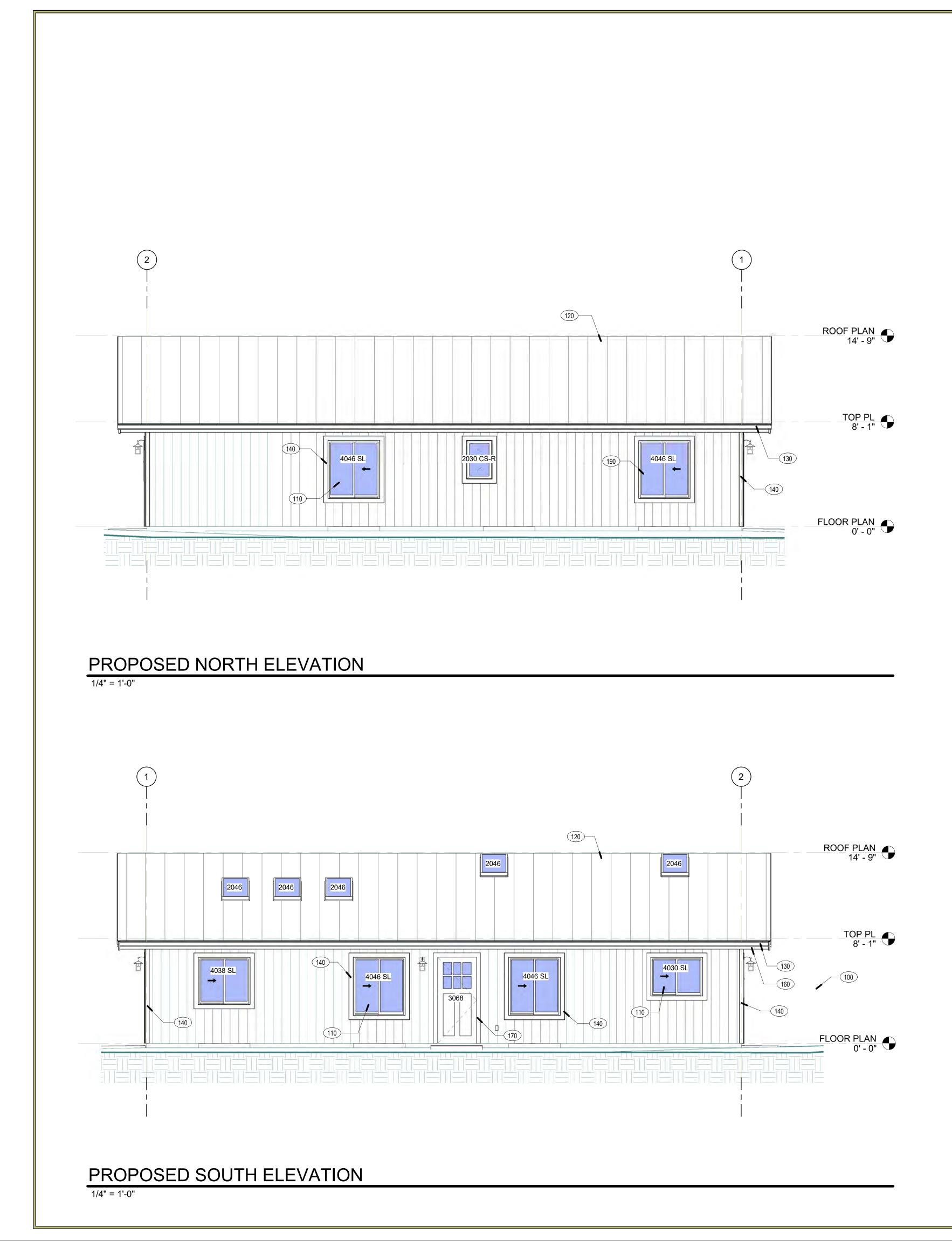
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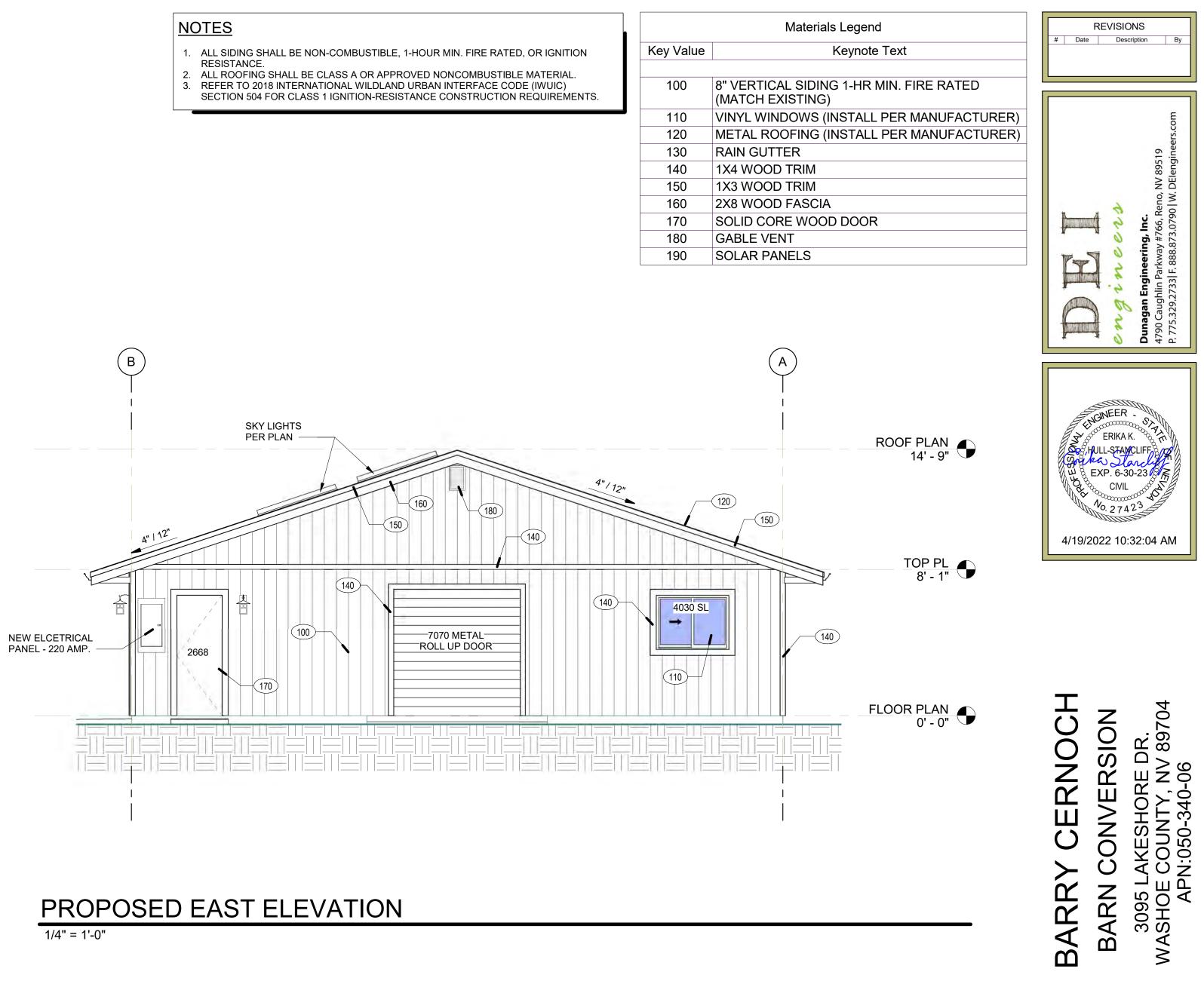
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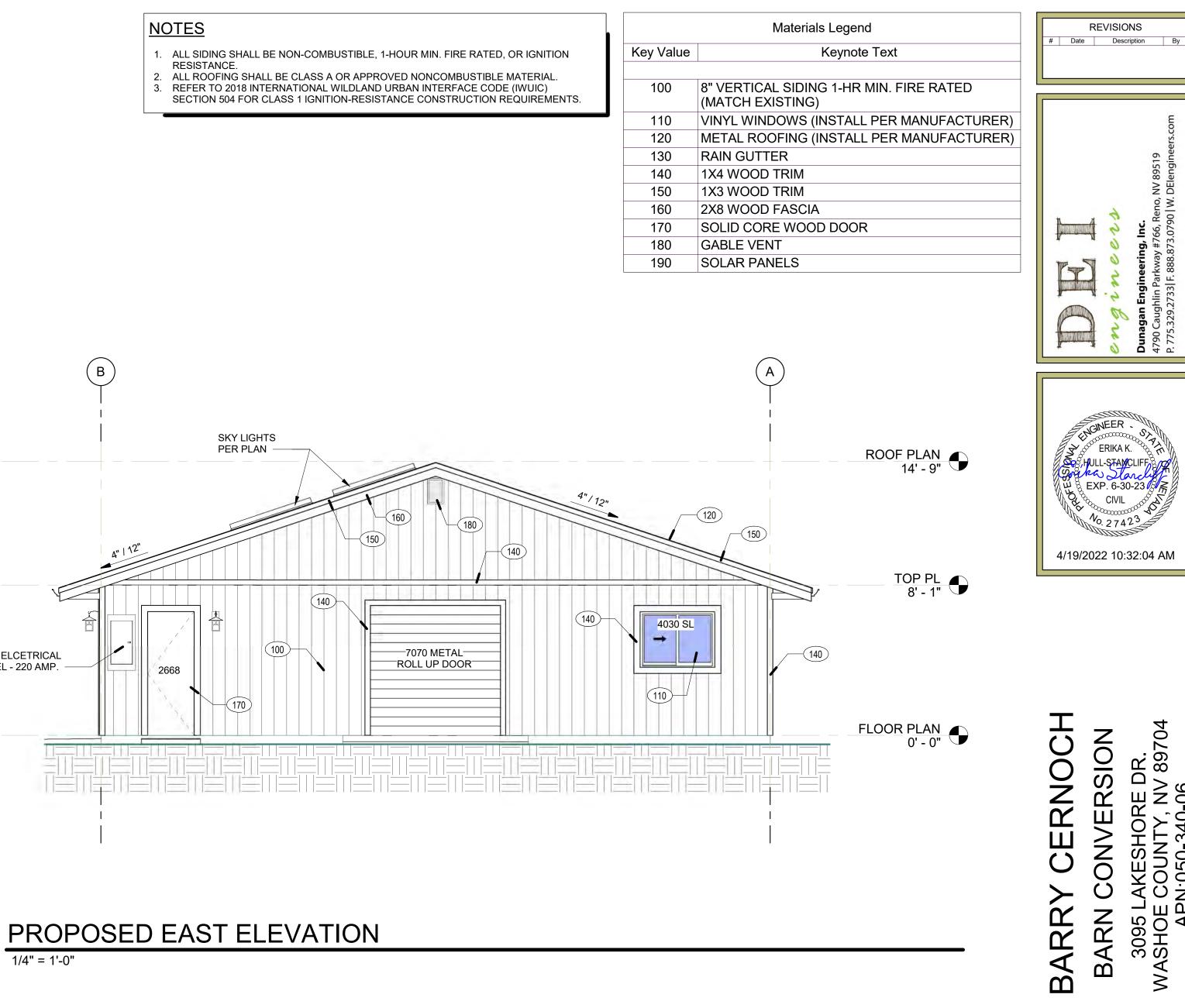


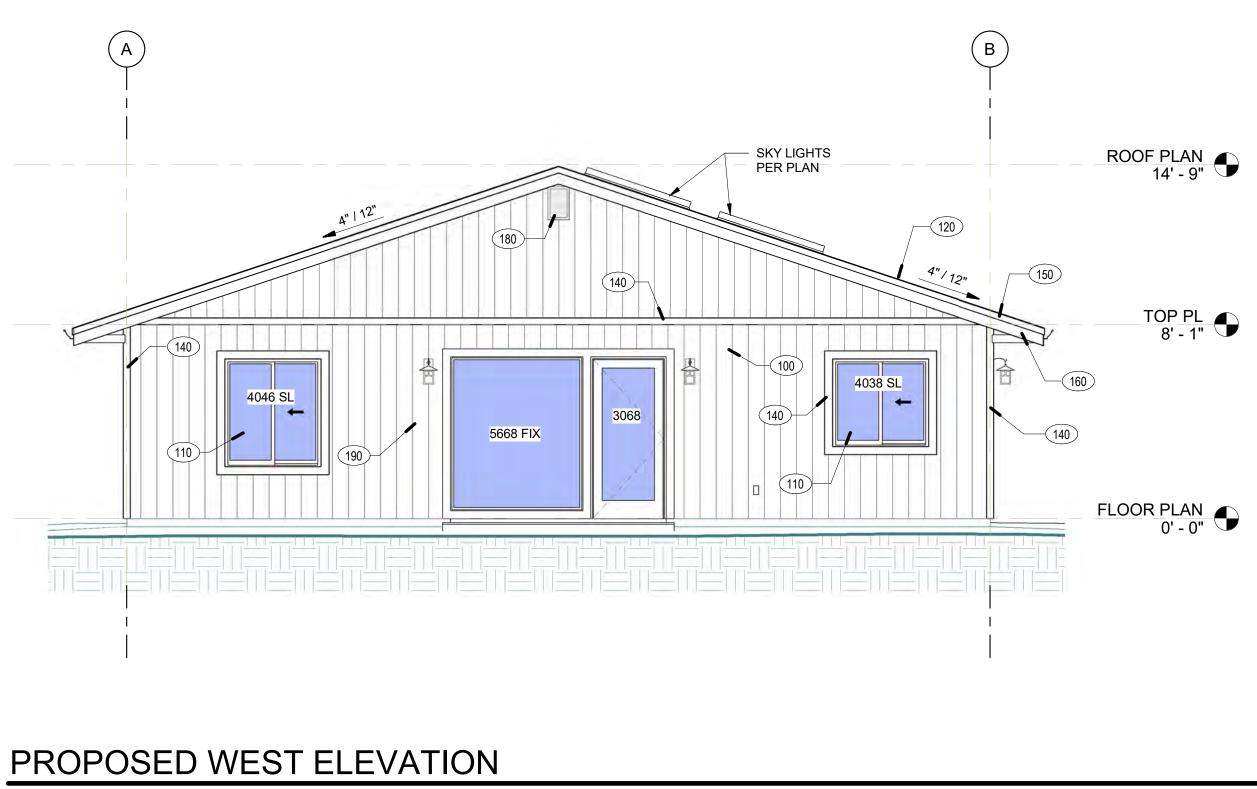






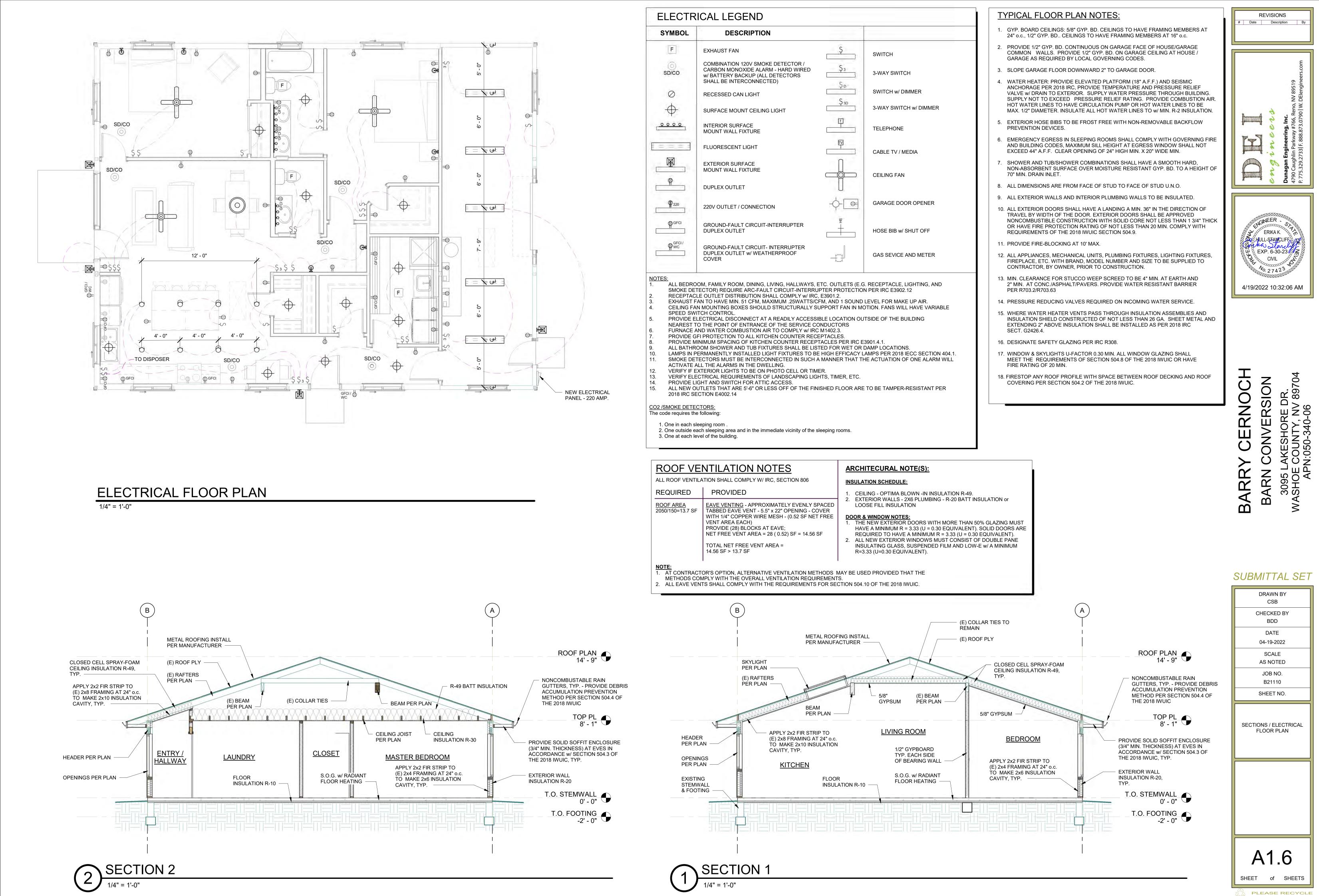






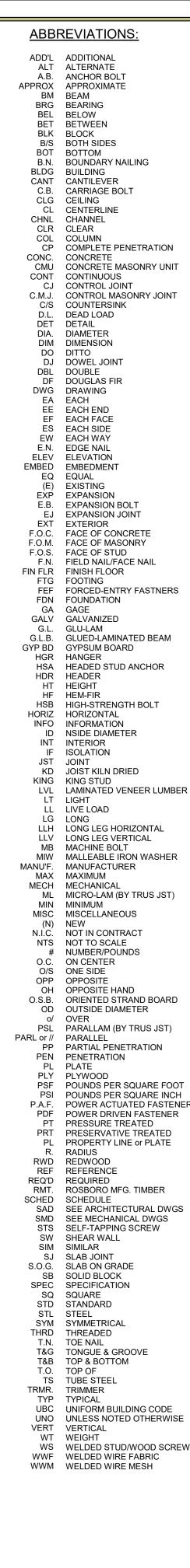
1/4" = 1'-0"

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DRAWN BY CSB
CHECKED BY BDD
DATE
04-19-2022
SCALE AS NOTED
JOB NO.
B21110
SHEET NO.
PROPOSED ELEVATIONS
A1.5
SHEET of SHEETS



EXHAUST FAN	~	
	2	SWITCH
COMBINATION 120V SMOKE DETECTOR / CARBON MONOXIDE ALARM - HARD WIRED w/ BATTERY BACKUP (ALL DETECTORS SHALL BE INTERCONNECTED)	\$3 	3-WAY SWITCH
RECESSED CAN LIGHT		SWITCH w/ DIMME
SURFACE MOUNT CEILING LIGHT	Ş _{3D}	3-WAY SWITCH w
INTERIOR SURFACE MOUNT WALL FIXTURE	Ţ	TELEPHONE
FLUORESCENT LIGHT		CABLE TV / MEDIA
EXTERIOR SURFACE MOUNT WALL FIXTURE		CEILING FAN
DUPLEX OUTLET	Ŭ	
220V OUTLET / CONNECTION	-\$-\$	GARAGE DOOR C
GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET	₩ +	HOSE BIB w/ SHU
GROUND-FAULT CIRCUIT- INTERRUPTER DUPLEX OUTLET w/ WEATHERPROOF COVER		GAS SEVICE AND
TECTOR) REQUIRE ARC-FAULT CIRCUIT-INTERRU LE OUTLET DISTRIBUTION SHALL COMPLY W/ IRC AN TO HAVE MIN. 51 CFM, MAXIMUM .25WATTS/C IN MOUNTING BOXES SHOULD STRUCTURALLY S ITCH CONTROL. LECTRICAL DISCONNECT AT A READILY ACCESS TO THE POINT OF ENTRANCE OF THE SERVICE CO AND WATER COMBUSTION AIR TO COMPLY W/ IRC SFI PROTECTION TO ALL KITCHEN COUNTER RECEP OOM SHOWER AND TUB FIXTURES SHALL BE LIS PERMANENTLY INSTALLED LIGHT FIXTURES TO BI TECTORS MUST BE INTERCONNECTED IN SUCH / ALL THE ALARMS IN THE DWELLING. EXTERIOR LIGHTS TO BE ON PHOTO CELL OR TIM ECTRICAL REQUIREMENTS OF LANDSCAPING LIG IGHT AND SWITCH FOR ATTIC ACCESS.	JPTER PROTECTION 5, E3901.2. FM, AND 1 SOUND I UPPORT FAN IN MC IBLE LOCATION OU ONDUCTORS M1402.3. EPTACLES. TACLES PER IRC ES TED FOR WET OR I E HIGH EFFICACY LA A MANNER THAT TH IER. HTS, TIMER, ETC.	N PER IRC E3902.12 LEVEL FOR MAKE UP DTION. FANS WILL HA TSIDE OF THE BUILD 3901.4.1. DAMP LOCATIONS. AMPS PER 2018 IECC IE ACTUATION OF ON
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ROOF VE	NTILATION NOTES	ARCHITECURAL
ALL ROOF VENTI	ATION SHALL COMPLY W/ IRC, SECTION 806	INSULATION SCHEDU
REQUIRED	PROVIDED	1. CEILING - OPTIMA
ROOF AREA 2050/150=13.7 SF	EAVE VENTING - APPROXIMATELY EVENLY SPACED TABBED EAVE VENT - 5.5" x 22" OPENING - COVER WITH 1/4" COPPER WIRE MESH - (0.52 SF NET FREE VENT AREA EACH) PROVIDE (28) BLOCKS AT EAVE; NET FREE VENT AREA = 28 (0.52) SF = 14.56 SF TOTAL NET FREE VENT AREA = 14.56 SF > 13.7 SF	 EXTERIOR WALLS LOOSE FILL INSUL DOOR & WINDOW NOT THE NEW EXTERION HAVE A MINIMUM REQUIRED TO HAY ALL NEW EXTERION INSULATING GLASS R=3.33 (U=0.30 EQ
METHODS CO	I TOR'S OPTION, ALTERNATIVE VENTILATION METHODS M MPLY WITH THE OVERALL VENTILATION REQUIREMENTS	S.



GENERAL NOTES AND SPECIFICATIONS:

DIVISION 1 - GENERAL:

- a. All work shall conform to the 2018 International Building Code (IBC) and applicable local codes. b. Where applicable allowable stresses have been increased 15% (Except Alpine and Placer Counties) for short duration and 60% for seismic and wind loading.
- c. Dunagan Engineering, Inc. is responsible for the structural items in the plans only. Should any changes be made, or should the results of these calculations not be fully or properly transferred to the plans by others, Dunagan Engineering, Inc. assumes no responsibility for the structure. No deviation from structural details shall be made without the written approval of the Structural Engineer. Approval by governing agency does not constitute authority to deviate from plans or
- specifications. d. All codes and standards shall be the most current edition as of the date of the calculations.
- e. The details shown on the drawings are typical. Similar details apply to similar conditions. f. The calculations are based upon a complete structure. Should an unfinished structure be subjected to loads, Dunagan Engineering, Inc. should be consulted for an interim design or if not, will assume no liability.
- g. Temporary supports, etc., are the sole responsibility of the framing contractor and have not been considered by the structural engineer. Framing contractor is responsible for the stability of the structure prior to the application of shear walls, roof and floor diaphrams and finish materials. He shall provide the necessary bracing to provide stability prior to the application of the
- aforementioned materials. Observation visits to the site by field representative of the Structural Engineer do not include inspections of construction means and methods. Observation performed by Architect and/or Structural Engineer during construction are not continuous and detailed inspection services are performed by others. Observations performed by Structural Engineer are performed solely for the purpose of determining if contractor understands design intent conveyed in the contract documents. Observations do not guarantee contractor's performance and are not to
- be construed as supervision of construction. h. Dunagan Engineering, Inc. expressly reserves its common law copyright and other property rights in these plans. These plans are not to be reproduced, changed or copied in any manner whatsoever, nor are to be assigned to a third party without first obtaining the written permission and consent of Dunagan Engineering, Inc. In the event of unauthorized reuse of these plans by a third party, the third party shall hold Dunagan Engineering, Inc. harmless
- These drawings and all written material herein are instruments of service and constitute original and unpublished work of the Engineer. They remain the property of the Engineer whether the project for which they are made be executed or not. They may not be duplicated, used on other projects or by other than the original Owner whose name appears herein without the express written consent of the Engineer.
- Adhesive anchors shall be Simpson AT-XP Epoxy per ESR-2508 with ASTM A36 threaded rod or approved equal, U.N.O., Expansion anchors shall be Simpson Strong Bolts per ESR-3037, U.N.O., Adhesive or expansion anchors shall not be installed without authorization by Structural Engineer and until concrete and masonry has cured to design strength.
- **DIVISION 2 FOUNDATION:**
- a. Building sites are assumed to be drained and free of clay or expansive soil. Any other conditions should be brought to the attention of Dunagan Engineering, Inc b. These calculations assume stable, undisturbed soils and level or stepped footings. Any other
- conditions should be reported to Dunagan Engineering, Inc.
- All footings shall bear on undisturbed soil with a footing depth 24" below frostline. d. All finish grade shall slope away from foundation for a minimum of 10'-0".
- e. An assumed soil bearing pressure is determined and will be increased in accordance with IBC Table 1806.2
- f. Fill material shall be free from debris, vegetation, and other foreign substances. g. Backfill trenches shall be compacted to 90% relative density per ASTM D1557 to within 12" of finished grade. The top 12" shall be landscape fill.
- h. Backfill at pipe trenches shall be compacted on both sides of pipe in 6" lifts.
- Waterproof exterior faces of all foundation walls adjacent to usable spaces. Waterproofing of all foundation and retaining walls to be the responsibility of the owner and/or contractor. All backfill against foundation walls must be compacted to 90% relative density, unless otherwise
- directed by a soils report. k. Perforated pipe sub-drain typical behind all retaining walls. Use 4" diameter PVC except where noted otherwise. Slope pipe to drain to daylight and drywell.
- **DIVISION 3 CONCRETE:** a. All concrete shall have a minimum 28 day compressive strength of 3000 psi. To accommodate the "Severe Weather for Concrete" category, concrete shall have a minimum 28 day compressive
- strength of 3000 psi for foundation walls and other vertical concrete exposed to weather and a minimum compressive strength of 3500 psi for slabs, porches and other exterior flatwork, including garage slabs, exposed to weather as recommended by Table R402.2 of the IRC and Section 1904.1 of the IBC. No Special Inspection is required as design assumes 2500 psi. b. Reinforcement shall be per ASTM A615 grade 60 ksi, U.N.O.
- Lap reinforcing Per Detail 5/S0.2, U.N.O.
- d. Reinforcement cover in cast-in-place concrete shall be as follows: (ACI Table 20.6.1.3.1) - 3" Concrete cast against and permanently exposed to earth. - 1 1/2" Concrete exposed to earth or weather with #5 bars or smaller.
- 0 3/4" Concrete not exposed to weather or in contact with ground, #11 bars and smaller, slabs, joists and walls.
- 1 1/2" Concrete not exposed to weather, beams, columns and pilaster, cover over ties. - 1 1/2" Clear to top for reinforcement in slabs on grade.
- e. All slabs on grade, S.O.G., shall have a minimum thickness of 4" and be reinforced with #3 at 18" o.c., or with Fibermesh as per manufacturers specifications equivalent to reinforcement specified above. U.N.O.
- Concrete shall be air-entrained to 6% +/- 1%. (For exterior slabs only) q. Provide slab control joints (saw cut or plastic inserts) at 10'-0" maximum spacing each way for 4" slab. Joint depth to be 1/4 of slab depth.

THESE NOTES APPLY TO ALL SHEETS:

- It shall be the contractor's direct responsibility to comply with typical details and general notes as delineated or defined on the typical detail drawings of these contract documents regardless of specific flagging or reference to applicable note or detail.
- It shall be the contractor's responsibility to coordinate with all trades regarding utilities passing through and under footings. Structural requirements for these conditions are delineated in typ. details.
- Top of footing elevations noted are minimum. See note 2 for additional requirements. Contractor to verify and coordinate all locations and sizes of openings in slabs, slab depressions, and curbs for all related construction prior to floor layout or construction. Contractor shall then use appropriate details or appropriate wall section for each applicable condition
- Contractor to verify dimensions with architect prior to construction Drawings are diagrammatic in nature and are not intended to indicate every opening or penetration in roof or other structure. Contractor shall coordinate and verify location and
- size of all such openings and penetrations with related subcontractors prior to roof or other framing layout or construction. Contractor shall then use appropriate typical or referenced details for each opening or penetration. Contractor to verify with appropriate sub-contractors the exact location, weight, and
- intended method of attachment of all items to be suspended from or in any way attached to any roof framing or other structural member unless such item(s) are clearly addressed by the structural construction documents. This information shall be transmitted in writing to structural engineer prior to final design or fabrication of structural framing members. Contractor to verify all existing conditions and dimensions and notify the architect in writing
- of any discrepancies.

SPECIAL INSPECTIONS AND DEFERRED SUBMITTALS:

- Special inspection, per the International Building Code chapter 17, AISC 360 and Table 1705.2.3 for steel and 1705.3 for concrete shall be required for the following types of work. See project Specifications for specified requirements:
- All concrete work for strengths greater than 2500 psi, except for slabs on grade, footings and non structural concrete. All reinforcing steel for concrete strengths greater than 2500 psi.
- All field welding (except metal studs, furring channels, etc.). Shop welding for procedures and multiple pass welds.
- All full penetration welds shall be specially inspected in accordance with AWS and the current International Building Code. All fillet welds shall be visually inspected in accordance with AWS and the current
- International Building Code. All masonry work, see notes under `MASONRY' for requirements. All masonry inspection shall also comply with the National Concrete Masonry Institute.
- Bolts installed in conc. or masonry. Does not include sill PL, anchor bolts and Holdown anchor bolts.
- All ASTM A-325 and/or ASTM A-490 High Strength Bolts
- All expansion bolts and adhesive anchors.
- All grouted dowels.

DIVISION 5 - METALS:

weather proofing methods may be used.

specifications, U.N.O.

- U.N.O.)
- the manufacturer.
- fabrication All welding shall conform to the American Welding Society specifications. All welding shall be
- standard cut washers.
- diameters from each end of the piece 2308.3.1.1 for alternate.

DIVISION 6 - WOOD:

- covering, U.N.O. b. Glu-Lams used for simple spans shall be 24F-V4 U.N.O. Glu-Lams used for continuous spans or cantilever shall be 24F-V8, U.N.O. Glu-Lams exposed to weather shall be rated for exterior use by manufacturer or approved protection from exposure to be provided. All plywood shall conform to APA DOC PS1 or DOC PS2. All shear plywood shall be C-D, C-C,
- 303 (T1-11), or approved equal. d. Where multiple trimmers or studs are specified, those trimmers are to be stacked in all wall framing and solid blocking to be provided at all floors down to the foundation e. Where posts with column caps, straps, or bearing plates are called for, the load is to be
- all floors, U.N.O.
- and/or soil to stud wall contact
- 3 1/2" wide and greater: Fb=2900 psi, Fv=290 psi, E=2,000,000 psi.
- ground to be preservative treated Douglas Fir.
- in accordance with ASTM A153. members of equal or better grade may be substituted.
- All floor openings shall be between joists, U.N.O. m. Do NOT notch beams, joists, and studs, U.N.O. n. Provide double joists below all parallel partition walls.
- o. No green lumber at time of covering shall be used on this project. p. No framing of any type shall be concealed prior to inspection by governing agencies. G. Sawn lumber shall have the
- all 4x12 & smaller fram - all 4x14, 4x16, 6x & 8x
- 4x4 posts all other posts and timl
- all 2x joists and rafters all 2x & 3x studs (unbr
- all 2x & 3x studs (unbra
- all 2x top plates
- all 2x and 3x sills manuf. truss components
- to be 2" t. All multiple studs to be attached with 16d's at 12" o.c.
- NAILS SEE NOTE #2 PANEL EDGES SEE NOTE #2 PANEL FIELD 1 ----2

HIP ROOF

ROOF SHEATHING:

FRAMING MATERIAL:

4x8 D.F. #2 at 2x4 walls or RMT U.N.O. Provide (2) Trimmers at openings greater than 4'-0" U.N.O. WALL FRAMING Existing 2x4 D.F. Stud or Construction Grade at 24" o.c. as occurs Typ. U.N.O.

- All insulating concrete.

a. All hardware called for shall be Simpson Strong-Tie Co, Inc. and installed per the manufacturer's

b. Structural steel shall conform to ASTM A992, grade 50 U.N.O. Miscellaneous steel such as plates, channels and angles may be ASTM A36. Steel pipe columns shall conform to ASTM A53, Type E or S. Steel tube sections shall conform to ASTM A500, Grade B. c. All steel exposed to weather shall be hot-dip galvanized after fabrication or other approved

d. Where finish is attached to steel provide 1/2" dia. bolt holes at 36" o.c., U.N.O.. For attachment of nailers see architectural drawings for finishes. (alternate 1/2" dia. x 3" nelson studs at 36" o.c.,

e. All grout under steel bearing plates shall be solid drypack or non-shrink grout placed as directed by

f. Shop drawings shall be submitted to the Structural Engineer for review and comment prior to

performed by certified welders approved by the local building authority. All shop welding shall be in an approved fabricator's shop authorized by the local building authority or special inspection per the IBC shall be provided. All field welding shall require special inspection per IBC Section 1704. All welding electrodes shall be E70XX or shielded wires with Fy = 70 ksi. All nails specified are common nails. No substitutions unless approved in writing by Dunagan

Engineering, Inc. or specifically addressed in these calculations or the plans. All nails exposed to weather shall be galvanized. Fasteners for pressure-preservative treated and fire-retardant treated wood shall be of hot-dipped zinc coated galvanized, stainless steel, silicon bronze or copper. The minimum nailing for all framing shall conform to IBC Table 2304.10.1. All bolts specified must meet ASTM A307. Bolt holes to be 1/32" to 1/16" larger than specified bolt.

Washers shall be used at each bolt head and nut next to wood. All washers to be not less than Wood plates or sills shall be bolted to the foundation or foundation wall. Steel bolts with a minimum

nominal diameter of 1/2" shall be used. Bolts shall be embedded at least 7 inches into the concrete or masonry. In a two pour system embedment shall be into the first pour. There shall be a minimum of two bolts per piece with one bolt located not more than 12 inches or less than 7 bolt

m. Plate washers a minimum of 3"x3"x1/4" thick shall be used on each bolt. See IBC section

a. All lumber framing shall be Douglas Fir Larch (DOC PS20) with moisture content < 19% at time of

transferred to the foundation with posts as specified in the plans and solid vertical grain blocking at f. All studs to be stud grade or better, U.N.O. In no instance shall a stud wall be used to resist lateral

pressures due to snow or soil. It is the owner and/or contractor's responsibility to eliminate snow

g. All laminated veneer lumber (LVL) and parallel strand lumber (PSL) specified shall have the following minimum design strengths: 1 3/4" wide: Fb=2600 psi, Fv=285 psi, E=1,900,000 psi.

h. All multiple-ply LVL members to be attached with (3) rows of 16d common nails at 12" o.c. for entire length of member. For a three piece member the nailing is from each side. i. Foundation sill plates, nailers, and ledgers in direct contact with concrete and within 6 1/2" of

Fasteners for preservative treated and fire treated wood shall be of hot dipped, zinc coated, galvanized steel, silicon, bronze or copper. The coating weights for zinc coated fasteners shall be

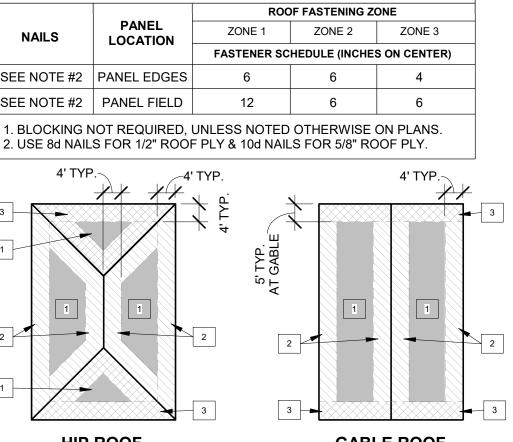
All framing members specified in these calculations and/or plans are minimums, and larger

all be concealed prior to inspection he following minimum grades (U.N.	O.):
ming members	#2
x framing members	#1
-	#2
nbers	#1
s	#2
raced length up to 10')	stud or construction
raced length exceeding 10')	#2
	standard
	standard

..grade per manuf r. All resawn and roughsawn beams are to be free of heart center.

s. Double joists shall be attached with (2) rows of 16d's at 12" o.c. edge distance of nailing

ROOF SHEATHING FASTENING SCHEDULE



GABLE ROOF

Existing 1/2" CDX APA Rated (32/16) or OSB equivalent, Apply face grain perpendicular to framing. Stagger panels and nail with 8d's common at 6" o.c. at edges and boundaries (blocking, drag trusses, shear blocks, etc.), and 12" o.c. field. U.N.O. (See special diaphragm nailing requirements this sheet)

DESIGN CRITERIA 2018 International Building Code (IBC) Local Building Department Standards Soil Bearing (IBC Table 1806.2)

WIND DESIGN DATA

Ultimate Design Wind Speed, Vu = 120 m.p.h. (3-Second Gust) Risk Category II Wind Importance Factor, Iw = 1.00

Wind Exposure C Internal Pressure Coefficient = +/- 0.18

Components & Cladding Design Pressures (ASCE 7 Section 30.4.2): a = 3.2 ft (ASCE 7 Figure 30.4-1)

Roof/Wall		Effective Wind Area	Design Wind Pressure, Pnet
Root/wall	Zone	(ft ²)	(psf)
	1	10	58.0
	4	20	58.0
1.0	1	50	35.2
2	1	100	18.0
0 3	2	10	84.5
õ	2	20	73.1
~	2	50	58.0
Roof > 20 to 27*	2	100	46.5
ě.	3	10	100.4
	3	20	86.0
	3	50	67.0
	3	100	52.6
1	4	10	34.0
	4	20	32.5
Wall	4	50	30.7
	4	100	29.3
3	5	10	42.0
	5 5 5	20	39.2
	5	50	35.5
	5	100	32.5

SEISMIC DESIGN DATA

Importance Factor, le = 1.00 (Risk Category II) Ss = 2.212 g and S1 = 0.781 g

Site class: = D

SDs = 1.474 g , SD1 = 0.885 g Seismic design category: = D

Basic seismic-force-resisting system(s): =

Light-Framed Walls Sheathed with Wood Structural Panels Rated for Shear Resistance. R = 6.5

N/S Design Base Shear (LRFD) = 9.1 kips

E/W Design Base Shear (LRFD) = 9.1 kips Cs (LRFD)= 0.2269

Analysis Procedure Used = Equivalent Lateral Force Procedure

<u>SNOW LOAD DATA:</u>		
Site Elevation	5048	FT
Ground Snow Load	Pg =	31
Flat-Roof Snow Load	Pf =	21
Snow Exposure Factor	Ce =	0.9
Snow Importance Factor	ls =	1.0
	•	

FLOOR FRAMING DESIGN LOADS Floor Live Load =

Thermal Factor

Floor Dead Load =

Total Floor Load =

40 PSF <u>S.O.G.</u> 40 PS

Ct = 1.1

ROOF LOADING:	TYPICAL
Snow Load =	21 PSF
Dead Load =	15 PSF
⁻otal Load =	36 PSF

FOOTING AND STEMWALL REQUIREMENTS

- 8" Wide w/ (1) #4 continuous top and #4 at 48" o.c. vertical, hook at footing (alternate hooks). Locate vertical at all Holdown Anchor Bolts. If top of stemwall exceeds 36" above top of footing, use #4 at 18" o.c. horizontal
- continuous and #4 at 24" o.c. vertical. All footings shall bear on undisturbed soil. Assumed soil bearing pressure
- is determined & increased in accordance w/ IBC Table 1806.2. • Exterior footings to be placed 24" below grade minimum, U.N.O.

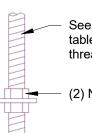
HOLDOWN INFORMATION See holdown schedule above and per plan.

SOILS & FOUNDATIONS:

Dunagan Engineering, Inc. has not made a geotechnical review of the building site and is not responsible for general site stability or soil suitability for the proposed project. A review by a geological engineer or qualified civil engineer may be desirable. Foundation design is based on minimum footing dimensions and bearing capacities set forth in Table 1806.2 of Chapter 18 in the 2018 International Building Code. Assume Class 5 soil with allowable soil bearing pressure of 1500 psf., with a constant expansion index less than 20. Footings shall extend 24" (minimum) below grade.

<u>CONN</u> Simpson SSTB16 SSTB24 HDU5-SD HDQ8-SE HHDQ11 HHDQ14 HD15 ABU44 ABU46 ABU66 ABU88 PB44 PB46 PB66 CBQ44 CBQ46 CBQ66 CB44 CB46

<u>THREADED ROD END</u> CONDITION AT HOLDOWNS



- See holdown specification table on this sheet for threaded rod size.

NECTIO	N CROSS R	EFERENCE					
n Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors
t Number	Product Number	Product Number	Product Number	Product Number	Product Number	Product Number	Product Number
6	STB16	CB66	KCB66	HU410	HD410	HGUS26-3	THDH26-3
4	STB24	CB68	KCB68	HU412	HD412	HGUS28-3	THDH28-3
SDS2.5	PHD5	HUCQ1.81/9-SDS	HDQ179IF	HU68	HD68	TJC37	SNP3
SDS3	UPHD8	HUCQ1.81/11-SDS	HDQ17112IF	HU610	HD610	THJA26	HJC26
11-SDS2.5	UPHD11		HDQ1714IF	HU612	HD612	MTHM	HJHC26
14-SDS2.5	UPHD14	HUCQ310-SDS	HDQ310IF	LSU26	LSSH15-TZ	DSC4R/L-SDS3	DSC4R/L
	TD15	HUCQ210-2-SDS	HDQ210-2IF	LSSU28		ST6224	KST224
	PAU44	HUCQ410-SDS	HDQ410IF	LSSU210	LSSH210	CS16	RS150
	PAU46	HUCQ412-SDS	HDQ412IF	SUR/L24	SKH24R/L	MSTC48B3	
	PAU66	HUCQ210-3-SDS	HDQ210-3IF	SUR/L26	SKH26R/L	H1	RT15
	PAU88	HUCQ5.25/9-SDS	HDQ5210IF	SUR/L210	SKH210R/L	H2.5A	RT7A
	WE44	HUCQ5.25/11-SDS	HDQ5212IF	IUS	THF	H2A	RT10
	WE46	HUCCQ610-SDS	HDQ610IF	HU11	HD17112	HGA10KT	HGA10
	WE66	HUCQ612-SDS	HDQ612IF	IUT	THF	A34	MP34
	KCBQ44	LUS24	JUS24	ITS	THO/TFL	A35	MPA1
	KCBQ46	LUS26	JUS26	ITT	THO/TFL	LTP4	MP4F
	KCBQ66	LUS28	JUS28	LUS26-2	JUS26-2	LS50	MP5
	KCB44	LUS210	JUS210	HHUS26-2	THD26-2	LS70	MP7
	KCB46	LUS46	JUS46	HGUS26-2	THDH26-2	LS90	MP9
	KCB48	HU46	HD46	HHUS28-2	THD28-2	CCQ/ECCQ	KCCQ/KECCQ

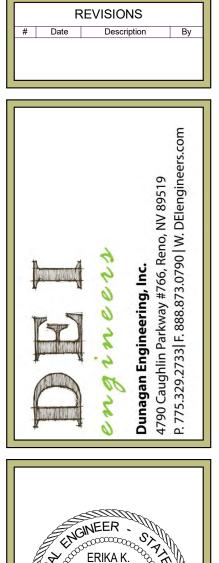
HOLDOWN SPECIFICATION TABLE

		(ALSO S	SEE SIMPSON STRONG	G-TIE CATAL	_OG)			
		POST		TI	HREADED ROD		SSTB BOLT	
		MIN.	SCREWS, BOLTS		EMBEDMENT			
HOLDOWN	CL	THICKNESS	OR NAILS	A.B. DIA.	8" STEM WALL	FOOTING	SGL. POUR	DBL. POUR
HTT4	1 5/16"	3"	(18) 16d's x 2 1/2"	5/8"	18"	-	SSTB24	SSTB24
HTT5	1 5/16"	3"	(26) 16d's x 2 1/2"	5/8"	24"	-	SSTB28	SSTB28
HDU5	1 5/16"	3"	(14) SDS 1/4"x2 1/2"	5/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	SSTB28	THRD. ROD OPTION ONLY
HDU8	1 3/8"	4 1/2"	(20) SDS 1/4"x2 1/2"	7/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HDQ8	1 1/4"	4 1/2"	(20) SDS 1/4"x3"	7/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HHDQ11	1 1/2"	5 1/2"	(24) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HHDQ14	1 1/2"	5 1/2"	(30) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HDU14	1 9/16"	5 1/2"	(36) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HD19	2 1/8"	5 1/2"x5 1/2"	(5) 1" DIA. BOLTS	1 1/4"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A

			NAI				NS				
NAIL TYPE	NOMINAL DIAMETER (GAGE)				NOMINA LENGTI	L N	/IN. EM	BED FO		MIN. NA	AIL LENGTH
6d COMMON	0	0.113" (11 ga.)			2"		1 3	8/8"			PLY. THICKNESS
8d COMMON	0.131" (10 1/4 ga.))	2 1/2"		13	1 3/8"				
10d COMMON	0.148" (9 ga.)			3"		13	3/4"				
12d COMMON	0.	0.148" (9 ga.)			3 1/2"			-	_		
16d COMMON	0.162" (8 ga.) 0.148" (9 ga.)			3 1/4"			-				
16d G.V. SINKER				3 1/4"			-			MIN. EMBED.	
DE	TERN	IINE	REQ'	D NA	IL DIA	METE		DLEN	<u>GTH</u>		
REQUIRED COMMO	N NAIL			8d					10d		
PLYWOOD THICKNE	SS	3/8"	1/2"	5/8"	3/4"	1 1/8"	3/8"	1/2"	5/8"	3/4"	1 1/8"
MINIMUM EMBEDME	ENT			1 3/8"				ı		1 3/4"	
MIN. NAIL LENGTH F	REQ'D	2"	2 1/8"	2 1/4"	2 3/8"	2 3/4"	2 1/8"	2 1/4"	2 3/8"	2 1/2"	2 7/8"
MIN. DIAMETER REQ'D 0.131" (10 1				' (10 1/4"	ga.)			0.148	" (10 1/4"	ga.)	

SHEET INDEX

- A0.0 COVER SHEET
- SITE PLAN A0.1
- A1.1 EXISTING FLOOR PLAN w/ DEMOLITION
- A1.2 EXISTING ELEVATIONS w/ DEMOLITION
- A1.3 PROPOSED FLOOR PLAN
- A1.4 PROPOSED ROOF PLAN
- PROPOSED ELEVATIONS A1.5
- SECTIONS / ELECTRICAL FLOOR PLAN A1.6
- GENERAL NOTES & TYPICAL DETAILS S0.1
- S0.2 TYPICAL DETAILS
- S0.3 TYPICAL DETAILS
- S0.4 DETAILS
- S1.1 FOUNDATION PLAN
- STRUCTURAL FLOOR PLAN S1.2
- **ROOF FRAMING PLAN / CEILING JOIST** S2.1



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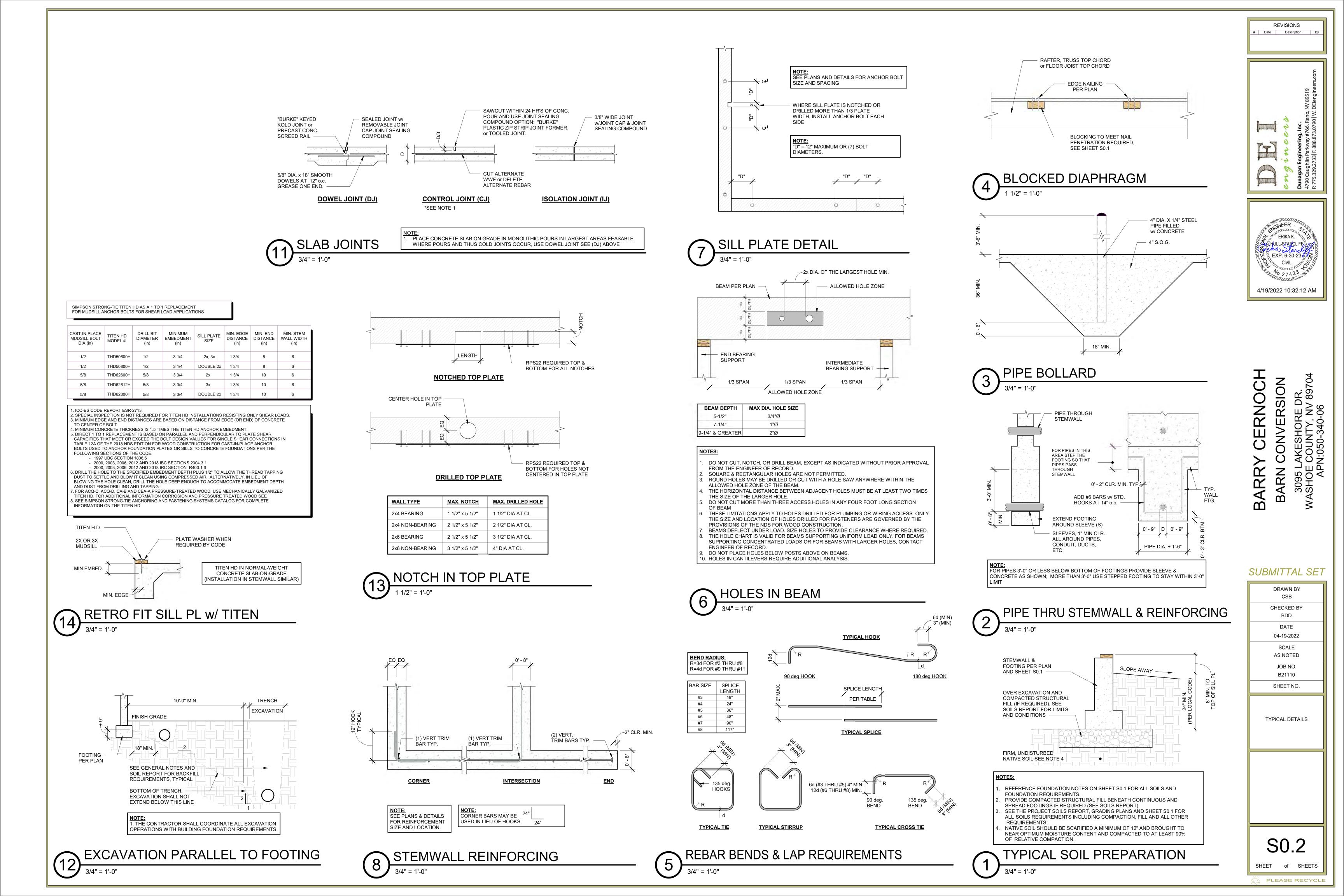
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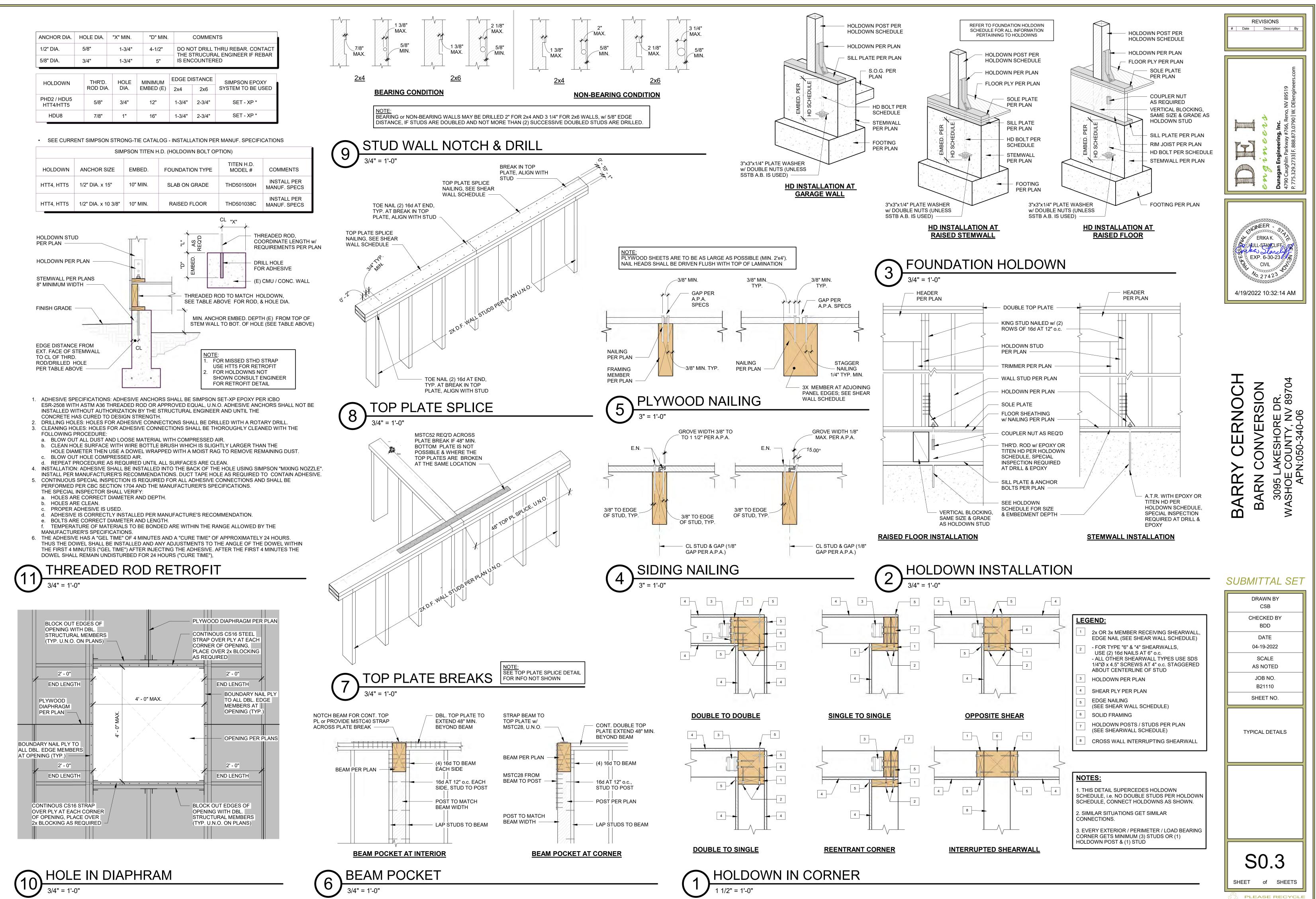


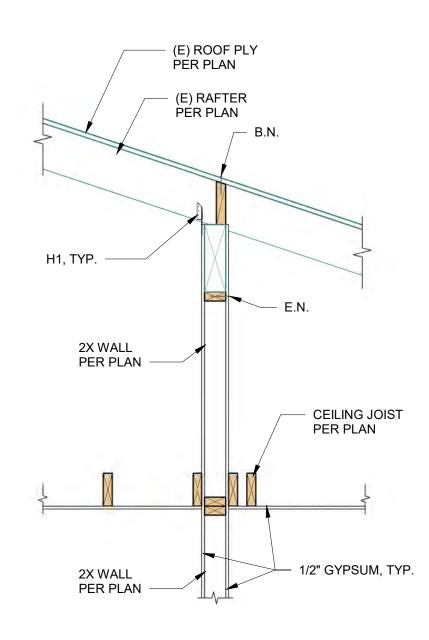
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DATE
04-19-2022
SCALE AS NOTED
JOB NO.
B21110
SHEET NO.
GENERAL NOTES & TYPICAL DETAILS
S0.1
SHEET of SHEETS

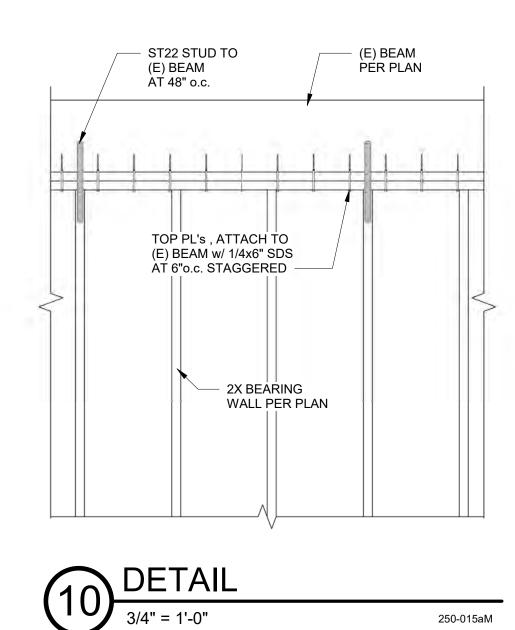
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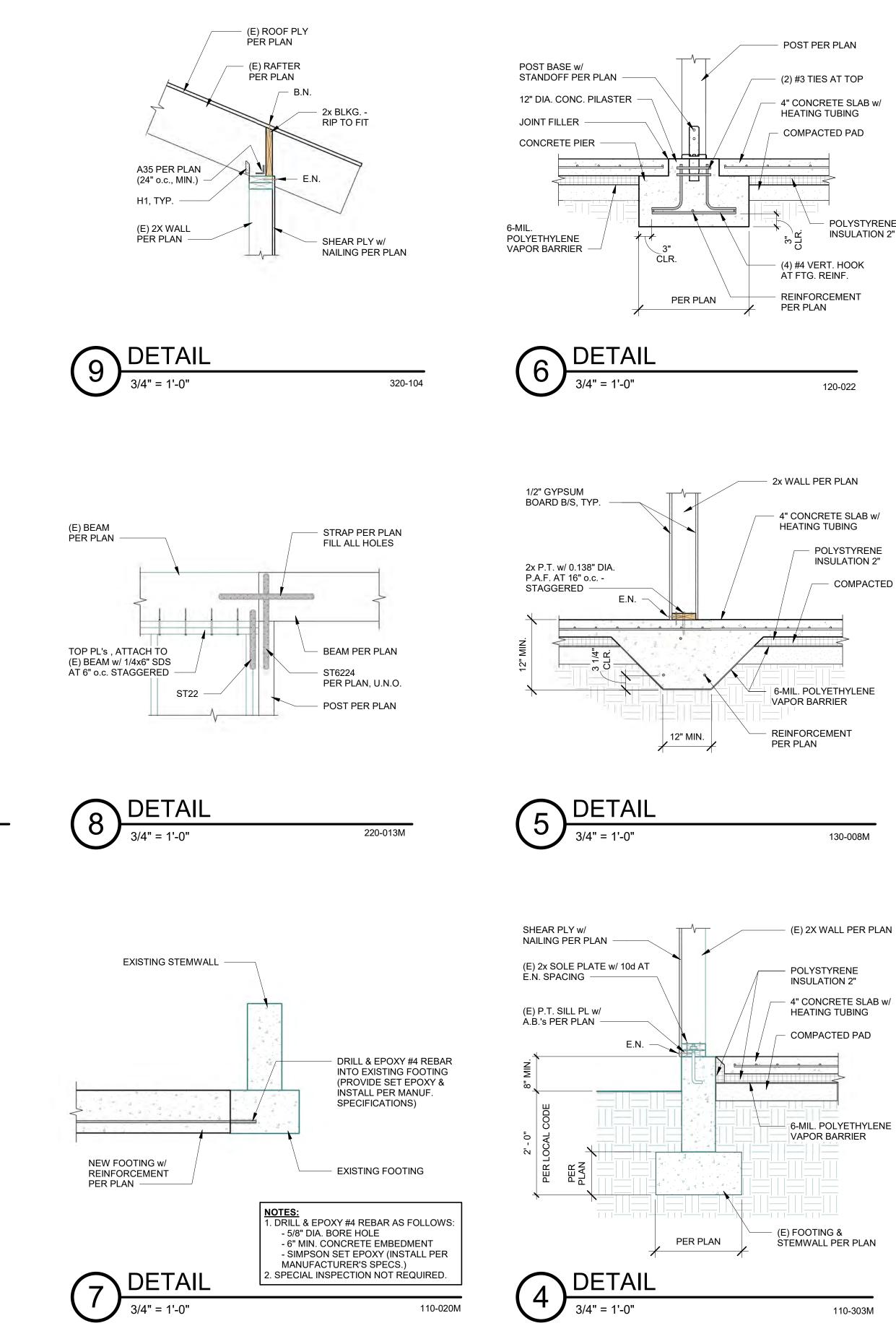


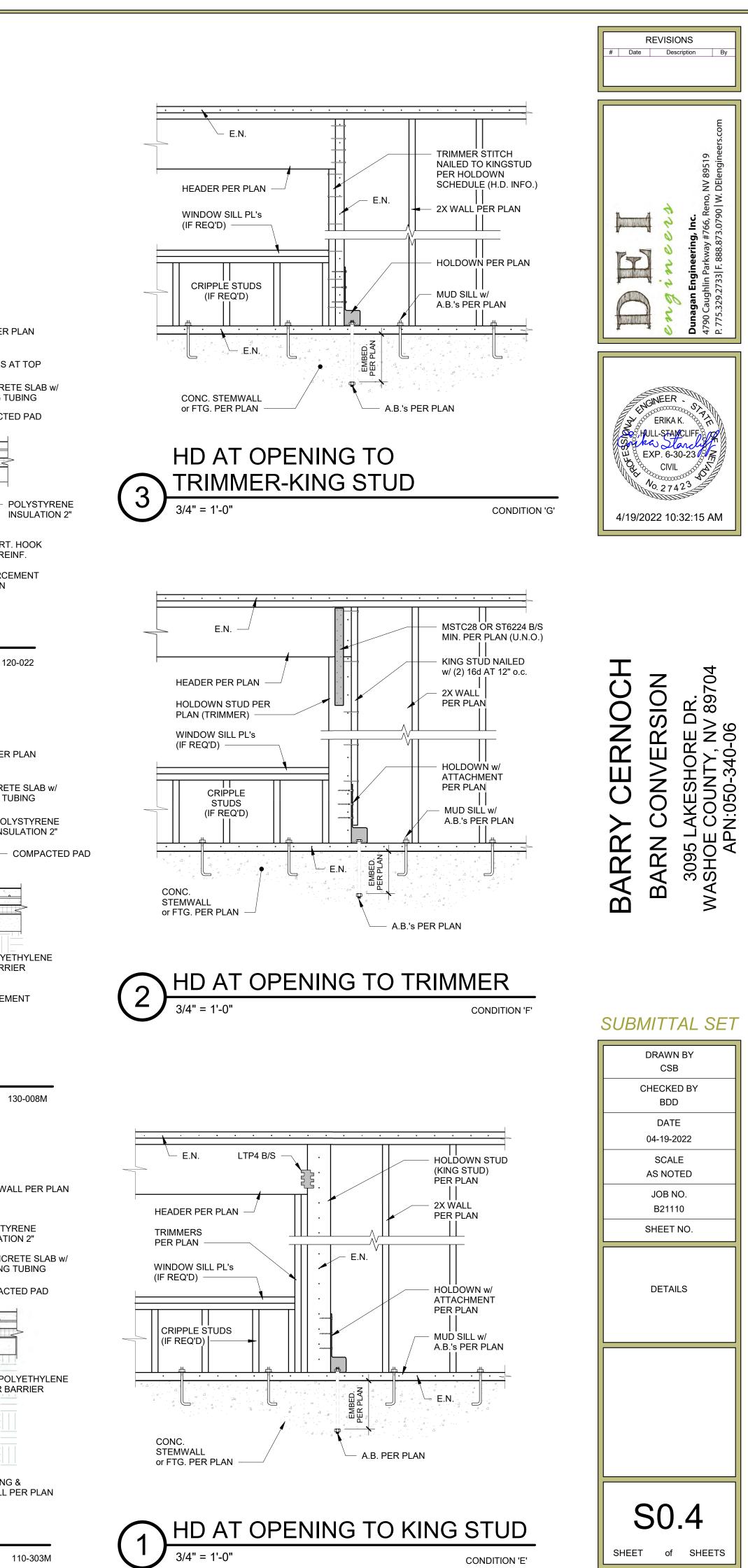




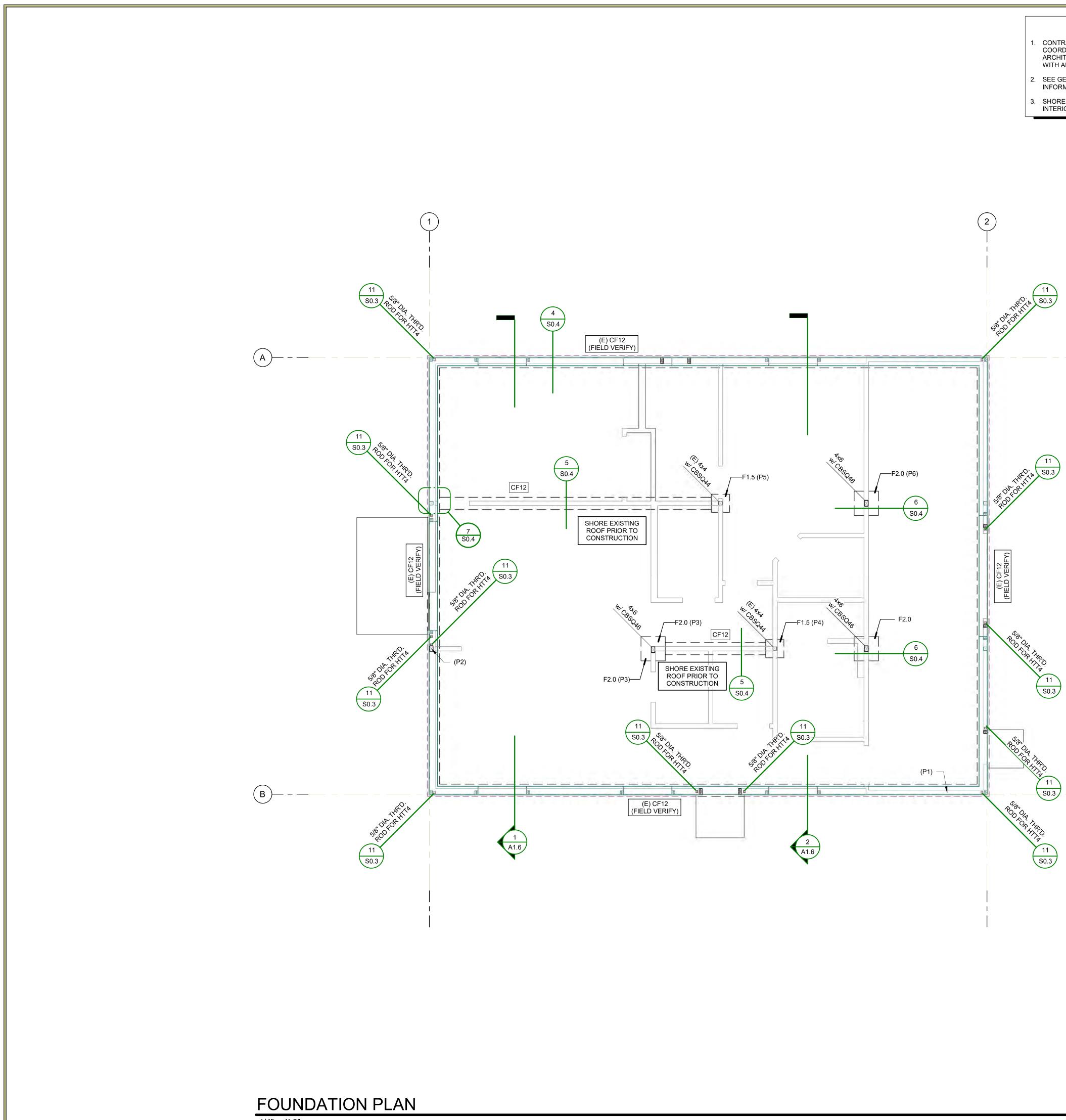








PLEASE RECYCLE



1/4" = 1'-0"

PLAN NOTES:

- . CONTRACTOR TO FIELD VERIFY ALL EXISTING CONDITIONS AND COORDINATE AND VERIFY ALL DIMENSIONS WITH THE ARCHITECTURAL DRAWINGS. CONTACT THE ENGINEER-OF-RECORD WITH ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
- INFORMATION.
- 3. SHORE EXISTING FRAMING AS REQUIRED TO INSTALL THE NEW INTERIOR CONTINUOS FOOTINGS AND 2x4 BEARING WALL.

SEE GENERAL NOTES AND STRUCTURAL DETAILS FOR BALANCE OF

FOUNDATION NOTES SILLS & PADS: EXISTING 2X PRESSURE TREATED LUMBER, TYP., U.N.O., TIMBERSTRAND LSL TREATED SILL PL'S PER ICC-ES ESR-1387.

ANCHOR BOLTS: 1/2" DIAMETER A.B. AT 4'-0" o.c. MAX., U.N.O. (2) ANCHOR BOLTS PER BOARD MIN., 12" FROM ENDS MAX. ANCHOR BOLTS EMBEDDED 7" MIN. INTO CONCRETE. SEE DETAIL 14/S0.2 FOR EXISTING CONCRETE CONDITIONS

DIMENSIONS: BUILDING DIMENSIONS SHOWN ARE FOR GENERAL REFERENCE ONLY. SEE THE ARCHITECTURAL DRAWINGS (S.A.D.) FOR ACTUAL BUILDING DIMENSIONS. ANY DISCREPANCIES ARE TO BE BROUGHT TO THE ATTENTION OF THE ENGINEER AND ARCHITECT SO CLARIFICATION CAN BE MADE. ALL DIMENSIONS SHALL BE VERIFIED BY THE CONTRACTOR AND SUBMITTED IN WRITING TO THE ENGINEER AND ARCHITECT FOR REVIEW PRIOR TO CONSTRUCTION.

NOTE: SEE STRUCTURAL FLOOR PLANS FOR LOCATION OF HOLDOWNS.

	PIER SC	CHEDUL	<u>.E</u>
MARK	<u>WIDTH</u> (each side)	<u>DEPTH</u>	<u>STEEL</u> (each way)
F1.5	18"	10"	(2) #4's
F1.75	21"	10"	(2) #4's
F2.0	24"	10"	(3) #4's
	<u>CONT. FOOT</u>	TING SC	HEDULE

SYMBOL	WIDTH	<u>DEPTH</u> (u.n.o.)	STEEL (continuous)
CF12	12"	8"	(2) #4's

8" WIDE STEMWALL w/ (1) #4 CONTINUOUS TOP AND #4 AT 48" o.c. VERTICAL, HOOK AT FOOTING (ALTERNATE HOOKS). IF THE TOP OF STEMWALL EXCEEDS 36" ABOVE THE TOP OF FOOTING, USE #4 AT 18" o.c. HORIZONTAL CONTINUOUS AND #4 AT 24" o.c VERTICAL.

- PROVIDE #4 VERTICALS AT 48" o.c. FOR TYPICAL STEM, HOOK AT FOOTING (ALTERNATE HOOKS).
- ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL, ASSUMED SOIL BEARING PRESSURE IS DETERMINED IN ACCORDANCE w/ IBC TABLE 1806.2, UNLESS SOIL REPORT IS PROVIDED.
- EXTERIOR FOOTINGS TO BE PLACED 24" BELOW GRADE PER APPLICABLE CODES.

NOTE: SEE DETAILS FOR SPECIAL REINFORCING OF STEMWALL AND FOOTINGS.

HC <u>holdown</u> htt4	DLDOWN SCHEDULE THREADED ROD- ANCHOR BOLT 5/8" DIA. w/ 18" EMBED INTO STEMWALL	NOTES HOLDOWN STUD (2) 2x-, U.N.O.	NOCH	RSION RE DR. NV 89704 .06
 PROVIDE (1) #4 H ANCHOR BOLTS HOLDOWN ANCH MUDSILL ANCHO USE RIM & BLOOO NAIL (2) 2x STUD 3" MIN. FROM EN 	TO BE SCREWED or NAILED TO HORIZONTAL AT TOP OF STEMW	ALL AT ALL HOLDOWN UPLIFT ONLY STANDARD ING PER PLAN). ING AT HOLDOWN HTT4. STAGGERED. LOCATE NAIL IN. EDGE DISTANCE.	BARRY CER	BARN CONVE 3095 LAKESHOR WASHOE COUNTY, APN:050-340-

SUBMITTAL SET

REVISIONS # Date Description By

GINEER .

ERIKA K. , HULL-STAMCLIFF

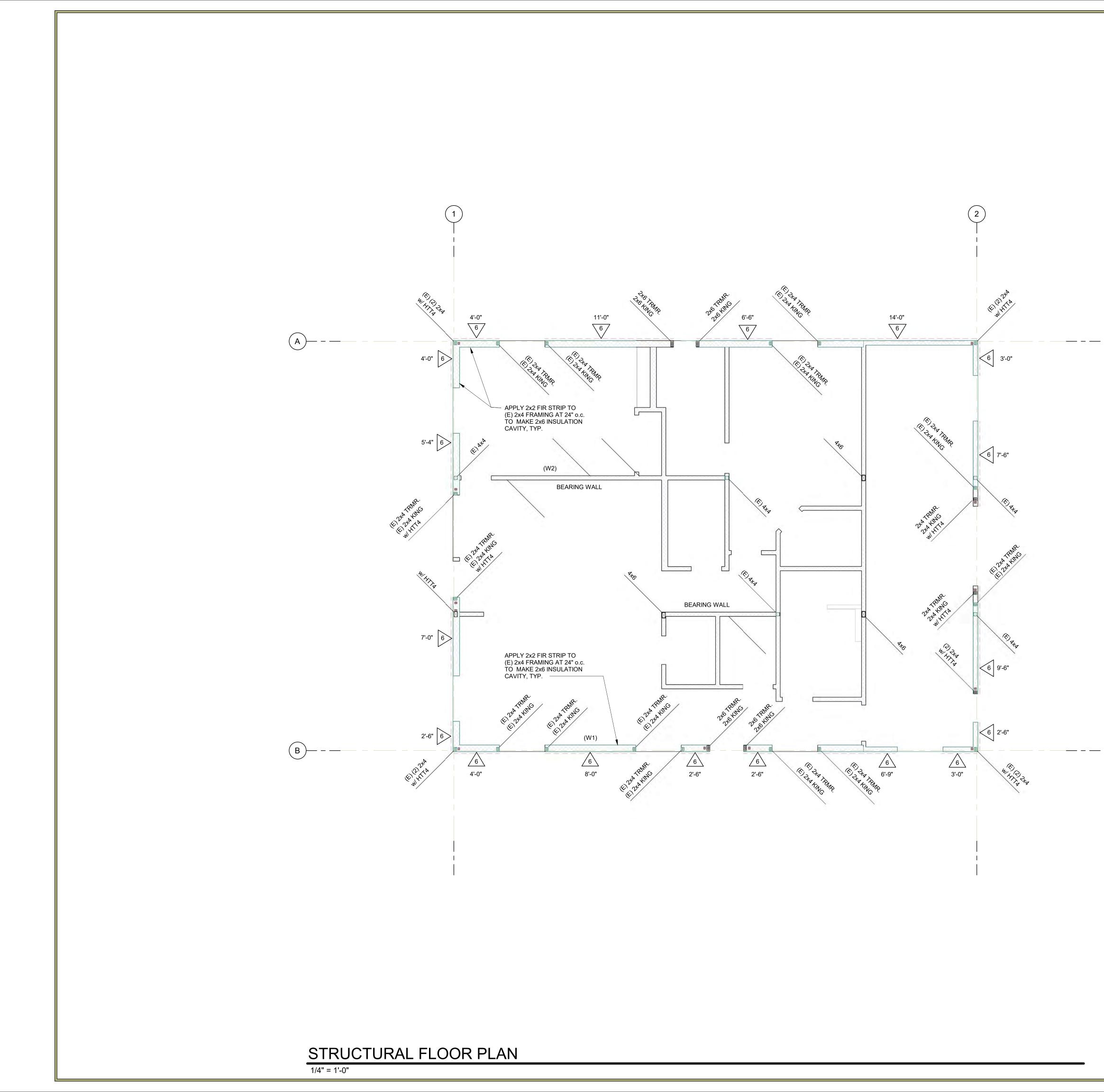
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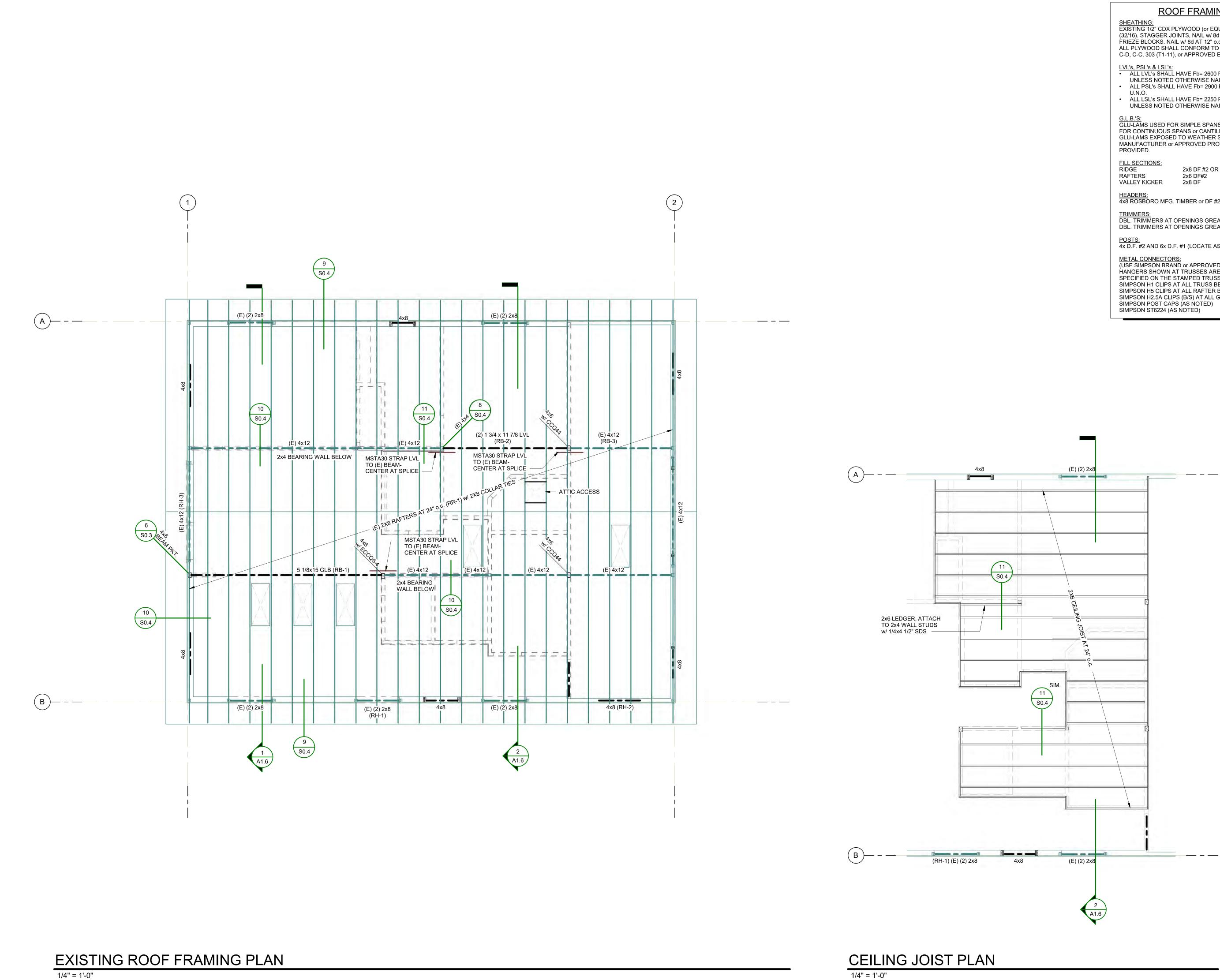
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CHECKED BY BDD
DATE 04-19-2022
SCALE AS NOTED
JOB NO. B21110
SHEET NO.
FOUNDATION PLAN
S1.1
SHEET of SHEETS

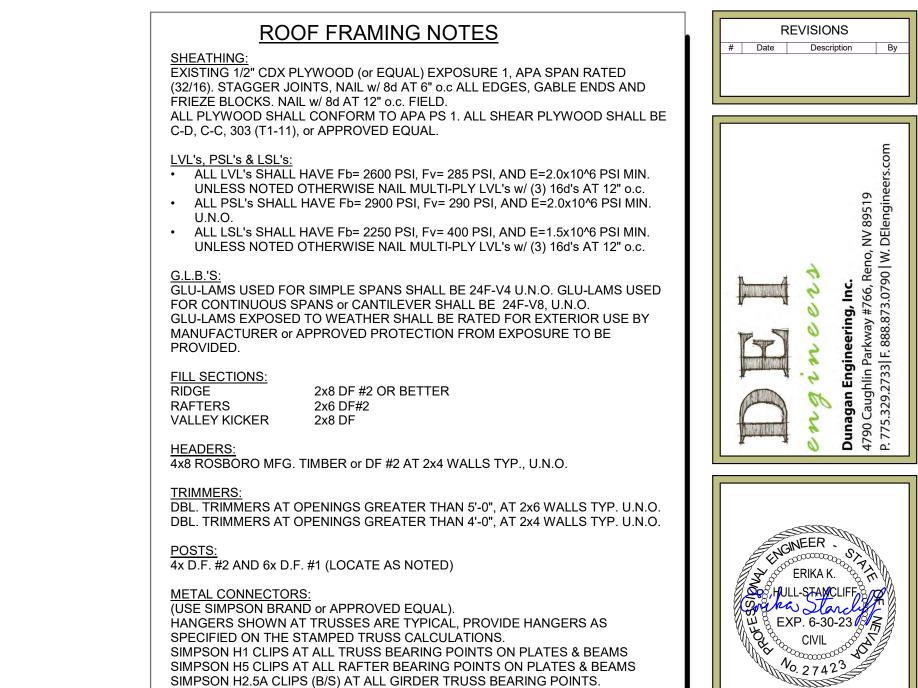


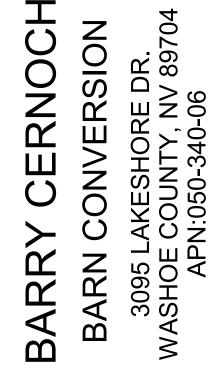
2	SHEAR W	ALL SCHE		ES	REVISIONS # Date Description By
<u>SYMBOL</u> ∧	SHEAR PLY	E.N. SPACING	3x STUDS AT ADJOINING PANEL EDGES	16d SPACING AT SHEAR <u>TRANSFER</u>	
$\boxed{6}$	3/8"	8d AT 6"	NO	6" o.c.	
4	3/8"	8d AT 4"	NO	4" o.c.	scom
Use 3/8" Sh Edge Nail A Use (12) 1 Use SIMPS Provide Blo	iear Ply, OSB, oi IT Top Plate, Mu 6d Nails AT All ⁻ ON MSTC52 To cking AT All Hor	ail AT 12" o.c., U.N Rated Equivalent I d Sill, All Posts, Sol Fop Plate Splices (6 Strap Top PL's Aci izontal Edges of Sh ing of plywood shea	U.N.O. le Plates, & All Si i0" Long) U.N.O. ross Breaks, U.N ear Plywood.		ひんしん しんしん しんしん しんしん しんしん しんしん いいしん いい
NOTE: Shear wall sche	dule includes all	shear options. See	plan for specific	requirements.	「「「」」」「「」」」」「「」」」「「」」」「「」」」「「」」」「」」」「」
					Line Part Hin Part 1333 [F. 2733] F. 2732] F. 2772] F. 7772] F. 7772] F. 7772] F. 7772] F. 7
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	HOLDO	VN SCHED		<u>ES</u>	
HOLDOV		THREADED ROD- ANCHOR BOLT		DOWN STUD	
HTT4	5/8	B" DIA. w/ 18" EMBI	1/1	2x-, U.N.O.	
HOLDOWN IN	FORMATION				—
		CREWED or NAILI			
ANCHOR I HOLDOWN	BÓLTS. N ANCHOR BOL	TS ARE DESIGNE	D FOR UPLIFT	ONLY STANDARD	
 USE RIM 8 NAIL (2) 2> 	& BLOCKING OF	S ARE REQUIRED R DOUBLE SOLID B THER w/ 16d's AT 4 TUDS AND PROVIE	BLOCKING AT H 4" o.c. STAGGEF	OLDOWN HTT4. RED. LOCATE NAILS	RSIO NV 897
* SEE HOLDO BOLTS.	WN ANCHOR B	OLT SCHEDULE S	HEET S0.1 FOR	SIMPSON SSTB	
-					
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	JOB NO. B21110
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	STRUCTURAL FLOOR PLAN
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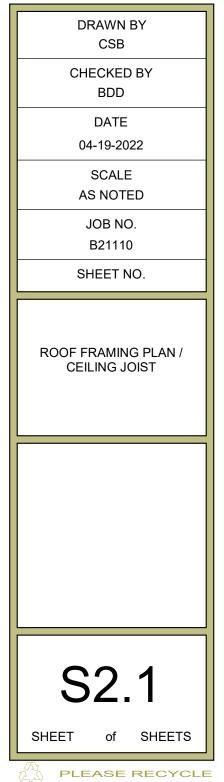


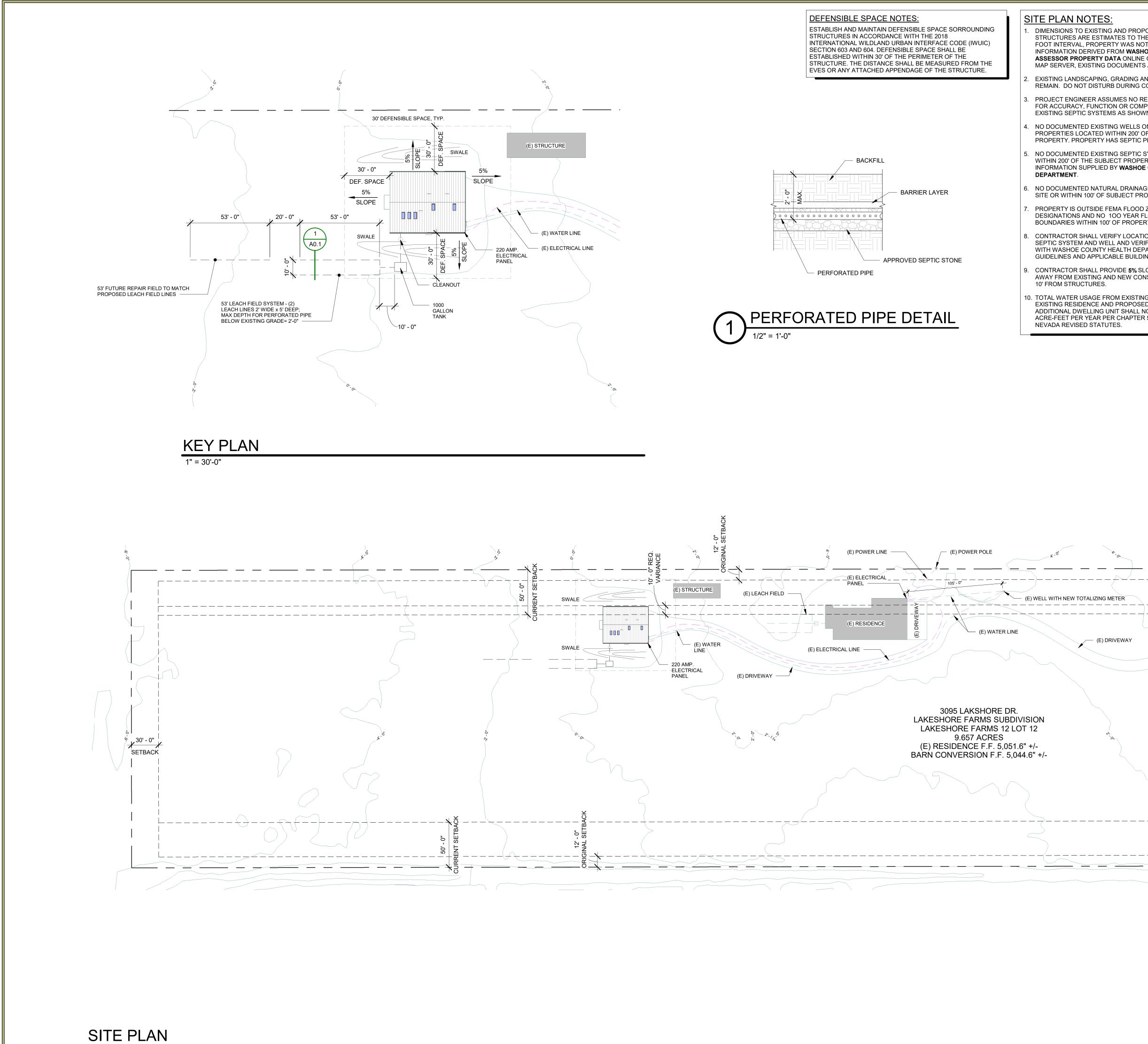
1/4" = 1'-0"





4/19/2022 10:32:21 AM





1" = 50'-0"

DIMENSIONS TO EXISTING AND PROPOSED STRUCTURES ARE ESTIMATES TO THE NEAREST ONE FOOT INTERVAL, PROPERTY WAS NOT SURVEYED. SITE INFORMATION DERIVED FROM WASHOE COUNTY ASSESSOR PROPERTY DATA ONLINE GIS INTERNET MAP SERVER, EXISTING DOCUMENTS AND SITE VISIT...

EXISTING LANDSCAPING, GRADING AND DRAINAGE TO REMAIN. DO NOT DISTURB DURING CONSTRUCTION.

PROJECT ENGINEER ASSUMES NO RESPONSIBILITY FOR ACCURACY, FUNCTION OR COMPLIANCE OF EXISTING SEPTIC SYSTEMS AS SHOWN ON THIS SITE.

NO DOCUMENTED EXISTING WELLS ON ADJACENT PROPERTIES LOCATED WITHIN 200' OF THE SUBJECT PROPERTY. PROPERTY HAS SEPTIC PER PLAN.

NO DOCUMENTED EXISTING SEPTIC SYSTEMS LOCATED WITHIN 200' OF THE SUBJECT PROPERTY BASED UPON INFORMATION SUPPLIED BY WASHOE COUNTY HEALTH

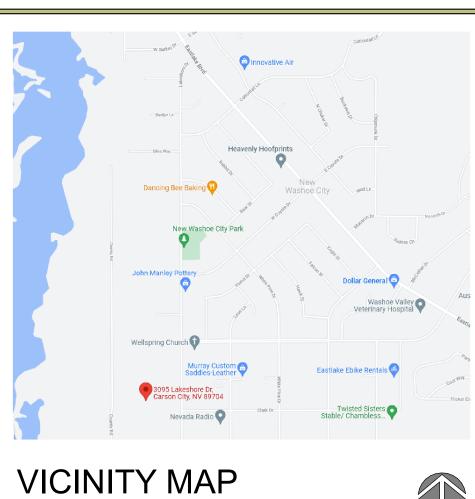
NO DOCUMENTED NATURAL DRAINAGES NOTED ON SITE OR WITHIN 100' OF SUBJECT PROPERTY.

PROPERTY IS OUTSIDE FEMA FLOOD ZONE DESIGNATIONS AND NO 100 YEAR FLOOD PLAIN BOUNDARIES WITHIN 100' OF PROPERTY.

CONTRACTOR SHALL VERIFY LOCATION OF EXISTING SEPTIC SYSTEM AND WELL AND VERIFY COMPLIANCE WITH WASHOE COUNTY HEALTH DEPARTMENT GUIDELINES AND APPLICABLE BUILDING CODES.

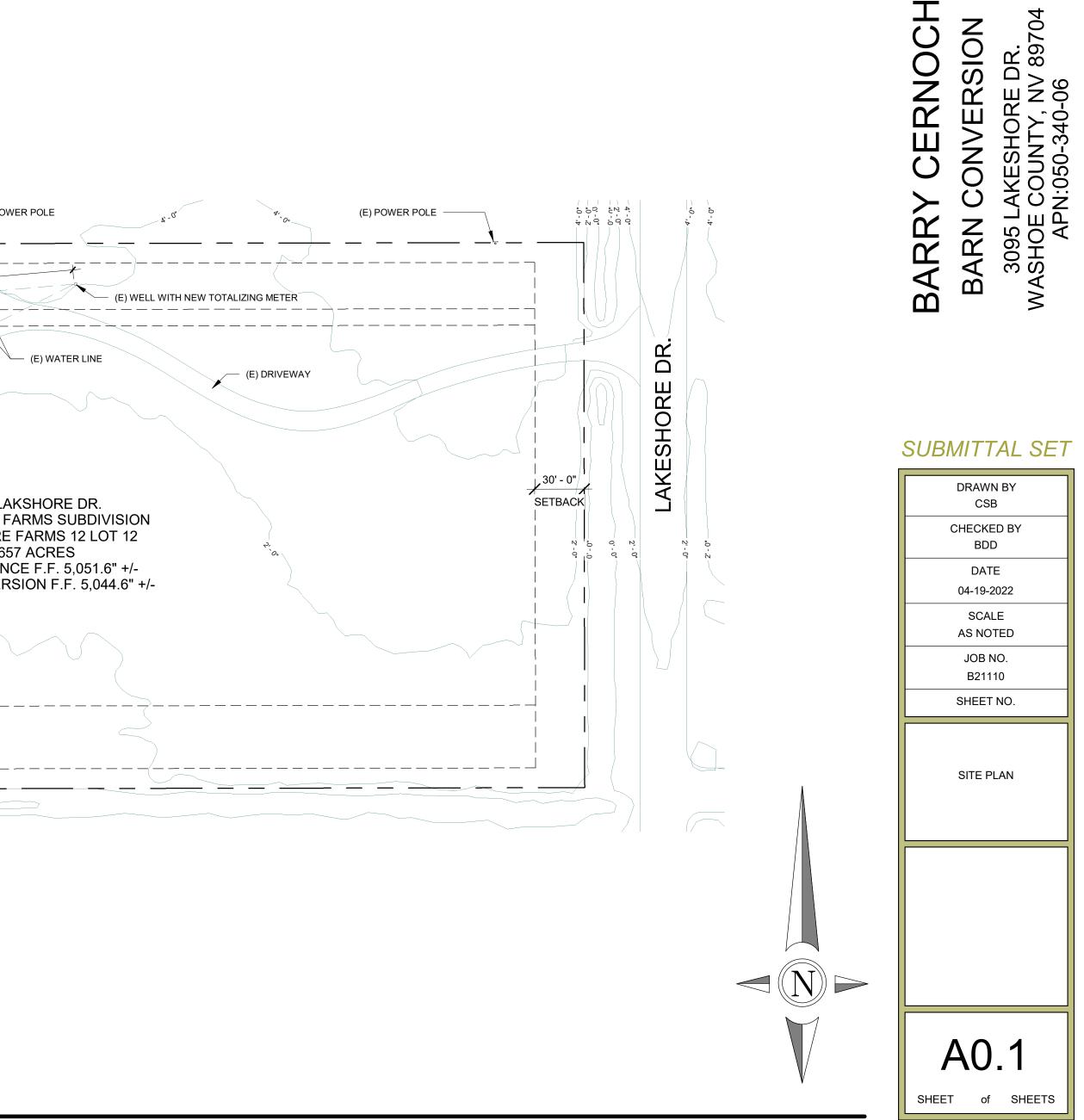
9. CONTRACTOR SHALL PROVIDE **5%** SLOPE DRAINAGE AWAY FROM EXISTING AND NEW CONSTRUCTION MIN.

10. TOTAL WATER USAGE FROM EXISTING WELL FOR EXISTING RESIDENCE AND PROPOSED DETACHED ADDITIONAL DWELLING UNIT SHALL NOT EXCEED 2 ACRE-FEET PER YEAR PER CHAPTER 534.180 OF NEVADA REVISED STATUTES.



N.T.S.





PLEASE RECYCLE