### Community Services Department Planning and Building DETATCHED ACCESSORY DWELLING ADMINISTRATIVE REVIEW APPLICATION



Community Services Department Planning and Building 1001 E. Ninth St., Bldg. A Reno, NV 89512-2845

Telephone: 775.328.6100

#### Washoe County Development Application

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Your entire application is a public record. If you have a concern about releasing personal information, please contact Planning and Building staff at 775.328.6100.

Project Information s		Staff Assigned Case No.:	
Project Name: Greves Garage Conversion			
Project Conversion of an existing garage to an additional dwelling unit with atta Description: garage. The existing garage contains a shop that will remain unchange		nit with attached n unchanged.	
Project Address: 1210 Mile Circle I	Drive, Reno, NV 89511		
Project Area (acres or square feet): 2205 sq. ft. (361sq. ft. of (E) shop, 1460 of proposed living space conversion, 383 sq. ft. of			nversion, 383 sq. ft. garage)
Project Location (with point of reference to major cross streets AND area locator):			
Mile Circle Drive	and Holcon	nb Ranch Lane, S	outh Reno
Assessor's Parcel No.(s):	Parcel Acreage:	Assessor's Parcel No.(s):	Parcel Acreage:
043-062-18	3.859		
Indicate any previous Washoe County approvals associated with this application: Case No.(s).			
Applicant Inf	ormation (attach	additional sheets if necess	sary)
Property Owner:		Professional Consultant: DEI Engineers	
Name: Ryan and Amanda Greves		Name: Erika K. Hull-Stancliff	
Address: 1210 Mile Circle Drive		Address: 1575 Delucchi Ln	
Reno, NV	Zip: 89511	Reno, NV	Zip: 89502
Phone: (510) 909-4611 Fax:		Phone: (775) 813-1591	Fax:
Email: ryan@eastbayfloorcovering.com		Email: erika@deiengineers.com	
Cell: Other:		Cell: (775) 813-1591 Other:	
Contact Person: Ryan Greves		Contact Person: Erika	
Applicant/Developer: Ryan Greves		Other Persons to be Contacted:	
Name: Ryan Greves		Name:	
Address: 1210 Mile Circle Drive		Address:	
Reno, NV	Zip: 89511		Zip:
Phone: (510) 909-4611 Fax:		Phone:	Fax:
Email: ryan@eastbayfloorcovering.com		Email:	
Cell: (510) 909-4611 Other:		Cell:	Other:
Contact Person: Erika		Contact Person:	
	For Office	Use Only	
Date Received:	Initial:	Planning Area:	
County Commission District:		Master Plan Designation(s):	
CAB(s):		Regulatory Zoning(s):	

#### Applicant Name: Ryan Greves

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

state of NEVADA ) county of Washoe ) I. Ryun Gneves

(please print name)

being duly sworn, depose and say that I am the owner\* of the property or properties involved in this application as listed below and that the foregoing statements and answers herein contained and the information herewith submitted are in all respects complete, true, and correct to the best of my knowledge and belief. I understand that no assurance or guarantee can be given by members of Planning and Building.

#### (A separate Affidavit must be provided by each property owner named in the title report.)

Assessor Parcel Number(s): 043-062-18

Printed Name Mile cincle PM Rewo Signed 1210 Address

Subscribed and sworn to before me this 30 day of November 2021

Notary Public in and for said county and state

My commission expires: 05-22 2023

\*Owner refers to the following: (Please mark appropriate box.)

- Owner 🗋
- Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- Dever of Attorney (Provide copy of Power of Attorney.)
- Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- D Property Agent (Provide copy of record document indicating authority to sign.)
- □ Letter from Government Agency with Stewardship

(Notary Stamp)



#### Applicant Name: Amanda Greves

The receipt of this application at the time of submittal does not guarantee the application complies with all requirements of the Washoe County Development Code, the Washoe County Master Plan or the applicable area plan, the applicable regulatory zoning, or that the application is deemed complete and will be processed.

STATE OF NEVADA

COUNTY OF WASHOE

Greves

(please print name)

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#### (A separate Affidavit must be provided by each property owner named in the title report.)

Assessor Parcel Number(s):\_043-062-18

Printed Name Signed Dr. Reno, N.V. Mile Address 121

Subscribed and sworn to before me this nod mg rol day of RICK for said county and state Notary Public in and My commission expires:

(Notary Stamp)



\*Owner refers to the following: (Please mark appropriate box.)

- Owner
- Corporate Officer/Partner (Provide copy of record document indicating authority to sign.)
- Dever of Attorney (Provide copy of Power of Attorney.)
- Owner Agent (Provide notarized letter from property owner giving legal authority to agent.)
- Property Agent (Provide copy of record document indicating authority to sign.)
- Letter from Government Agency with Stewardship

#### Administrative Review Permit Application for a Detached Accessory Dwelling Supplemental Information

(All required information may be separately attached)

1. What is the size (square footage) of the main dwelling or proposed main dwelling (exclude size of garage)?

5010 square feet (existing main dwelling)

2. What is the size of the proposed detached accessory dwelling (exclude size of garage)? If a manufactured or modular home is the secondary dwelling, list the age and size of the unit.

2205 sq. ft. (361sq. ft. of (E) shop, 1460 of proposed living space conversion, 383 sq. ft. garage)

3. How are you planning to integrate the main dwelling and secondary dwelling to provide architectural compatibility of the two structures?

The existing garage to be converted to living space matches the architectural style of the existing main dwelling.

5. How many off-street parking spaces are available? Parking spaces must be shown on site plan. Are any new roadway, driveway, or access improvements be required?

parking spaces. least street New Roadways niveway OR impa

6. What will you do to minimize any potential negative impacts (e.g. increased lighting, removal of existing vegetation, etc.) your project may have on adjacent properties?

Existing vegetation and light mitigation to remain and will suffice.

7. Is the subject property part of an active Home Owners Association (HOA) or Architectural Control Committee?

🗅 Yes 🛛 📓 No	If yes, please list the HOA name.
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8. Are there any restrictive covenants, recorded conditions, or deed restrictions (CC&Rs) that may prohibit a detached accessory dwelling on your property?

Yes No If yes, please attach a copy.	Yes	No No	lf yes, please attach a copy.
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9. Only one accessory dwelling unit, whether attached or detached, is allowed per parcel. Is there a guest apartment, mother-in-law unit, next-gen addition with kitchen or any other type of secondary dwelling on the subject property?

Yes No If yes, please provide information on the secondary unit.	
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10. List who the service providers are for the main dwelling and accessory dwelling:

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	Main Dwelling	Accessory Dwelling
Sewer Service	Municipal	Municipal
Electrical Service	NV Energy	NV Energy
Solid Waste Disposal Service	Waste Management	Waste Management
Water Service	Well	Well



#### SITE PLAN NOTES:

- 1. DIMENSIONS TO EXISTING AND PROPOSED STRUCTURES ARE ESTIMATES TO THE NEAREST ONE FOOT INTERVAL, PROPERTY WAS NOT SURVEYED. SITE INFORMATION DERIVED FROM **WASHOE COUNTY ASSESSOR PROPERTY DATA** ONLINE GIS INTERNET MAP SERVER, EXISTING DOCUMENTS AND SITE VISIT..
- 2. EXISTING LANDSCAPING, GRADING AND DRAINAGE TO REMAIN. DO NOT DISTURB DURING CONSTRUCTION.
- 3. NO DOCUMENTED EXISTING SEPTIC SYSTEMS LOCATED WITHIN 200' OF THE SUBJECT PROPERTY BASED UPON INFORMATION SUPPLIED BY **WASHOE COUNTY HEALTH DEPARTMENT**.
- 4. NATURAL DRAINAGE DITCH ADJACENT TO PROPERTY AS NOTED ON SITE PLAN.
- 5. PROPERTY IS WITHIN FEMA FLOOD BOUNDARY AND MUST COMPLY WITH THE WASHOE COUNTY FLOOD HAZARD REDUCTION ORDINANCE. ALL MECHANICAL AND ELECTRICAL SHALL BE ELEVATED TO A MINIMUM OF 24" ABOVE HIGHEST ADJACENT GRADE. PROVIDE FLOOD VENTS PER PLAN.
- 6. CONTRACTOR SHALL VERIFY LOCATION OF EXISTING WELL.
- CONTRACTOR SHALL PROVIDE 5% SLOPE DRAINAGE AWAY FROM EXISTING AND NEW CONSTRUCTION MIN. 10' FROM STRUCTURES.



# GREVES GARAGE CONVERSION 1210 MILE CR. RENO, NV 89511 APN: 043-062-18







- 1. GYP. BOARD CEILINGS: 5/8" GYP. BD. CEILINGS TO HAVE FRAMING MEMBERS AT 24" o.c., 1/2" GYP.BD. CEILINGS TO HAVE FRAMING MEMBERS AT 16" o.c.
- 2. PROVIDE 1/2" GYP.BD. CONTINUOUS ON GARAGE FACE OF HOUSE/GARAGE COMMON WALLS. PROVIDE 1/2" GYP.BD. ON GARAGE CEILING AT HOUSE / GARAGE AS REQUIRED BY LOCAL GOVERNING CODES.
- 3. SLOPE GARAGE FLOOR DOWNWARD 2" TO GARAGE DOOR.
- 4. WATER HEATER: PROVIDE ELEVATED PLATFORM (18" A.F.F.) AND SEISMIC ANCHORAGE PER 2018 IRC, PROVIDE TEMPERATURE AND PRESSURE RELIEF VALVE W/DRAIN TO EXTERIOR. SUPPLY WATER PRESSURE THROUGH BUILDING. SUPPLY NOT TO EXCEED PRESSURE RELIEF RATING. PROVIDE COMBUSTION AIR. HOT WATER LINES TO HAVE CIRCULATION PUMP OR HOT WATER LINES TO BE MAX. 1/2" DIAMETER.INSULATE ALL HOT WATER LINES TO W/MIN. R-2 INSULATION.
- 5. EXTERIOR HOSE BIBS TO BE FROST FREE WITH NON-REMOVABLE BACKFLOW PREVENTION DEVICES.
- 6. EMERGENCY EGRESS IN SLEEPING ROOMS SHALL COMPLY WITH GOVERNING FIRE AND BUILDING CODES, MAXIMUM SILL HEIGHT AT EGRESS WINDOW SHALL NOT EXCEED 44" A.F.F. CLEAR OPENING OF 24" HIGH MIN. X 20" WIDE MIN.
- 7. SHOWER AND TUB/SHOWER COMBINATIONS SHALL HAVE A SMOOTH HARD, NON-ABSORBENT SURFACE OVER MOISTURE RESISTANT GYP. BD. TO A HEIGHT OF 70" MIN. DRAIN INLET.
- 8. ALL DIMENSIONS ARE FROM FACE OF STUD TO FACE OF STUD U.N.O.
- 9. ALL EXTERIOR WALLS AND INTERIOR PLUMBING WALLS TO BE INSULATED. 10. ALL EXTERIOR DOORS SHALL HAVE A LANDING A MIN. 36" IN THE DIRECTION OF TRAVEL BY WIDTH OF THE DOOR.
- 11. PROVIDE FIRE-BLOCKING AT 10' MAX.
- 12. ALL APPLIANCES, MECHANICAL UNITS, PLUMBING FIXTURES, LIGHTING FIXTURES, FIREPLACE, ETC. WITH BRAND, MODEL NUMBER AND SIZE TO BE SUPPLIED TO CONTACTOR, BY OWNER, PRIOR TO CONSTRUCTION.
- 13. PRESSURE REDUCING VALVES REQUIRED ON INCOMING WATER SERVICE.
- 14. WHERE WATER HEATER VENTS PASS THROUGH INSULATION ASSEMBLIES AND INSULATION SHIELD CONSTRUCTED OF NOT LESS THAN 26 GA. SHEET METAL AND EXTENDING 2" ABOVE INSULATION SHALL BE INSTALLED AS PER 2018 IRC SECT. G2426.4.
- 15. DESIGNATE SAFTY GLAZING PER IRC R308.
- 16. WINDOW U-FACTOR 0.30 MIN.

A1.2

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PROVIDE (2) 16"x8" VENTS, (1) ON EACH SIDE OF THE BUILDING, IN THE GARAGE.



REVISIONS

Description PLAN CHECK 
 Date
 Description
 By

 05-20-21
 PLAN CHECK
 KMD

 11-12-21
 REVISIONS
 KMD



DRAWN BY KML
CHECKED BY EHS
DATE
4-23-21
SCALE AS NOTED
JOB NO. BB20121
SHEET NO.
(E) FLOOR PLAN w/ DEMO AND PROPOSED FLOOR PLAN
A1.1
SHEET of SHEETS









	TYPICAL FLOOR PLAN NOTES:	REVISIONS
	<ol> <li>GYP. BOARD CEILINGS: 5/8" GYP. BD. CEILINGS TO HAVE FRAMING MEMBERS AT 24" o.c., 1/2" GYP.BD. CEILINGS TO HAVE FRAMING MEMBERS AT 16" o.c.</li> </ol>	#         Date         Description         By           1         05-20-21         PLAN CHECK         KMD           2         11-12-21         REVISIONS         KMD
	<ol> <li>PROVIDE 1/2" GYP.BD. CONTINUOUS ON GARAGE FACE OF HOUSE/GARAGE COMMON WALLS. PROVIDE 1/2" GYP.BD. ON GARAGE CEILING AT HOUSE / GARAGE AS REQUIRED BY LOCAL GOVERNING CODES.</li> </ol>	
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	5. EXTERIOR HOSE BIBS TO BE FROST FREE WITH NON-REMOVABLE BACKFLOW PREVENTION DEVICES.	<b>ng, Inc.</b> 873.079
	6. EMERGENCY EGRESS IN SLEEPING ROOMS SHALL COMPLY WITH GOVERNING FIRE AND BUILDING CODES, MAXIMUM SILL HEIGHT AT EGRESS WINDOW SHALL NOT EXCEED 44" A.F.F. CLEAR OPENING OF 24" HIGH MIN. X 20" WIDE MIN.	in Parkwa 733] F. 888
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$\chi_{1}$	16. WINDOW U-FACTOR 0.30 MIN.	

# CONVERSION 1210 MILE CR. RENO, NV 89511 APN: 043-062-18 RAGE GА GREVES

DRAWN BY KML
CHECKED BY EHS
DATE
SCALE AS NOTED
JOB NO. BB20121
SHEET NO.
PROPOSED EAST/WEST ELEVATIONS
A1.3
SHEET of SHEETS

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- 15. DESIGNATE SAFTY GLAZING PER IRC R308.
- 16. WINDOW U-FACTOR 0.30 MIN.

#### ARCHITECTURAL NOTE(S):

**INSULATION SCHEDULE:** 

- 1. CEILING OPTIMA BLOWN-IN INSULATION R-49
- 2. (E) EXTERIOR WALLS R-19 BATT INSULATION MIN. (FIELD VERIFY)
- 3. GARAGE AND BATHROOM/SHOP PARTITION WALL R-20 INSULATION, MIN.

#### DOOR & WINDOW NOTE(S):

- THE NEW EXTERIOR DOORS WITH MORE THAN 50% GLAZING MUST HAVE A MINIMUM U FACTOR = 0.30. SOLID DOORS ARE REQUIRED TO HAVE A MINIMUM U FACTOR = 0.30.
- ALL NEW EXTERIOR WINDOWS MUST CONSIST OF DOUBLE PANE INSULATING GLASS, SUSPENDED FILM AND LOW-E w/ A MINIMUM U FACTOR = 0.30.





### **PROPOSED SOUTH ELEVATION** 1/4" = 1'-0"



#### PROPOSED NORTH ELEVATION 1/4" = 1'-0"









### REFLECTED CEILING PLAN 1/4" = 1'-0"



MMMMMMMMM.



				REVISIONS
ELECTR	ICAL LEGEND			#         Date         Description         By           1         05-20-21         PLAN CHECK         KMD
SYMBOL	DESCRIPTION			
F	F/L EXHAUST FAN w/ LIGHT	Ş	SWITCH	com Scom
O SD/CO	COMBINATION 120V SMOKE DETECTOR / CARBON MONOXIDE ALARM - HARD WIRED w/ BATTERY BACKUP (ALL DETECTORS SHALL BE INTERCONNECTED)	 	3-WAY SWITCH	NV 89519 DElengineer
$\oslash$	RECESSED CAN LIGHT		TELEPHONE	80 W. I
<b></b>	SURFACE MOUNT CEILING LIGHT		THERMOSTAT	<b>ng, Inc.</b> .873.079
$\square \bigcirc \bigcirc$	INTERIOR SURFACE MOUNT WALL FIXTURE		DOOR BUTTON	ngineeri 33] F. 888
	FLUORESCENT LIGHT			gan Ei aughli
	EXTERIOR SURFACE MOUNT WALL FIXTURE		CABLE TV / MEDIA	<b>Duna</b> P. 775.3
 	DUPLEX OUTLET		CEILING FAN	CINEER .
∯ gFCI	GROUND-FAULT CIRCUIT-INTERRUPTER DUPLEX OUTLET		GARAGE DOOR OPENER	The solution of the solution o
	GROUND-FAULT CIRCUIT- INTERRUPTER DUPLEX OUTLET w/ WEATHERPROOF COVER			₩ EXP. 6-30-23
<u>NOTES:</u> 1. ALL BED SMOKE	ROOM, FAMILY ROOM, DINING, LIVING, HALLWAY DETECTOR) REQUIRE ARC-FAULT CIRCUIT-INTEF	/S, ETC. OUTLETS (I RRUPTER PROTECT	E.G. RECEPTACLE, LIGHTING, AND FION PER IRC E3902.12	11/23/2021 10:58:12 AM
2. RECEPT 3. EXHAUS 4. CEILING	ACLE OUTLET DISTRIBUTION SHALL COMPLY w/ T FAN TO HAVE MIN. 51 CFM, MAXIMUM .25WATT FAN MOUNTING BOXES SHOULD STRUCTURALL	IRC, E3901.2. S/CFM, AND 1 SOUI Y SUPPORT FAN IN	ND LEVEL FOR MAKE UP AIR. MOTION. FANS WILL HAVE VARIABLE	
5. PROVID	SWITCH CONTROL. E ELECTRICAL DISCONNECT AT A READILY ACCE IT TO THE POINT OF ENTRANCE OF THE SERVICE	SSIBLE LOCATION	OUTSIDE OF THE BUILDING	
<ul> <li>6. FURNACE AND WATER COMBUSTION AIR TO COMPLY w/ IRC M1402.3.</li> <li>7. PROVIDE GELPROTECTION TO ALL KITCHEN COUNTER RECEPTACLES.</li> </ul>				
8. PROVID 9. ALL BAT	E MINIMUM SPACING OF KITCHEN COUNTER REC HROOM SHOWER AND TUB FIXTURES SHALL BE	CEPTACLES PER IRC	C E3901.4.1. DR DAMP LOCATIONS.	ΙŌ
10. LAMPS I 11. SMOKF	N PERMANENTLY INSTALLED LIGHT FIXTURES TO DETECTORS MUST BE INTERCONNECTED IN SUC	D BE HIGH EFFICAC	Y LAMPS PER 2018 IECC SECTION 404.1.	
ACTIVA 12 VERIEY	TE ALL THE ALARMS IN THE DWELLING.	TIMER	· · · · · · · · · · · · · · · · · · ·	
13. VERIFY	ELECTRICAL REQUIREMENTS OF LANDSCAPING	LIGHTS, TIMER, ETC	C.	μ
<ul> <li>14. PROVIDE LIGHT AND SWITCH FOR AT HE ACCESS.</li> <li>15. ALL NEW OUTLETS THAT ARE 5'-6" OR LESS OFF OF THE FINISHED FLOOR ARE TO BE TAMPER-RESISTANT PER 2018 IRC SECTION E4002.14</li> </ul>				

### <u>CO2 /SMOKE DETECTORS:</u> The code requires the following:

One in each sleeping room .
 One outside each sleeping area and in the immediate vicinity of the sleeping rooms.
 One at each level of the building.

GARAGE CON 1210 MILE CR. RENO, NV 89511 APN: 043-062-18 GREVES

DRAWN BY KML
CHECKED BY EHS
DATE 4-23-21
SCALE AS NOTED
JOB NO. BB20121
SHEET NO.
REFLECTED CEILING PLAN AND ELECTRICAL PLAN
A1.5
SHEET of SHEETS



#### **GENERAL NOTES AND SPECIFICATIONS:**

**DIVISION 1 - GENERAL** 

- a. All work shall conform to the 2018 International Building Code (IBC) and applicable local codes. b. Where applicable allowable stresses have been increased 15% (Except Alpine and Placer Counties) for short duration and 60% for seismic and wind loading.
- c. Dunagan Engineering, Inc. is responsible for the structural items in the plans only. Should any changes be made, or should the results of these calculations not be fully or properly transferred to the plans by others, Dunagan Engineering, Inc. assumes no responsibility for the structure. No deviation from structural details shall be made without the written approval of the Structural Engineer. Approval by governing agency does not constitute authority to deviate from plans or
- specifications. d. All codes and standards shall be the most current edition as of the date of the calculations.
- e. The details shown on the drawings are typical. Similar details apply to similar conditions. f. The calculations are based upon a complete structure. Should an unfinished structure be subjected to loads, Dunagan Engineering, Inc. should be consulted for an interim design or if not, will assume no liability.
- g. Temporary supports, etc., are the sole responsibility of the framing contractor and have not been considered by the structural engineer. Framing contractor is responsible for the stability of the structure prior to the application of shear walls, roof and floor diaphrams and finish materials. He shall provide the necessary bracing to provide stability prior to the application of the
- aforementioned materials. Observation visits to the site by field representative of the Structural Engineer do not include inspections of construction means and methods. Observation performed by Architect and/or Structural Engineer during construction are not continuous and detailed inspection services are performed by others. Observations performed by Structural Engineer are performed solely for the purpose of determining if contractor understands design intent conveyed in the contract documents. Observations do not guarantee contractor's performance and are not to
- be construed as supervision of construction. h. Dunagan Engineering, Inc. expressly reserves its common law copyright and other property rights in these plans. These plans are not to be reproduced, changed or copied in any manner whatsoever, nor are to be assigned to a third party without first obtaining the written permission and consent of Dunagan Engineering, Inc. In the event of unauthorized reuse of these plans by a third party, the third party shall hold Dunagan Engineering, Inc. harmless
- These drawings and all written material herein are instruments of service and constitute original and unpublished work of the Engineer. They remain the property of the Engineer whether the project for which they are made be executed or not. They may not be duplicated, used on other projects or by other than the original Owner whose name appears herein without the express written consent of the Engineer.
- Adhesive anchors shall be Simpson AT-XP Epoxy per ESR-2508 with ASTM A36 threaded rod or approved equal, U.N.O., Expansion anchors shall be Simpson Strong Bolts per ESR-3037, U.N.O., Adhesive or expansion anchors shall not be installed without authorization by Structural Engineer and until concrete and masonry has cured to design strength.
- **DIVISION 2 FOUNDATION:** a. Building sites are assumed to be drained and free of clay or expansive soil. Any other conditions should be brought to the attention of Dunagan Engineering, Inc.
- b. These calculations assume stable, undisturbed soils and level or stepped footings. Any other conditions should be reported to Dunagan Engineering. Inc.
- All footings shall bear on undisturbed soil with a footing depth 24" below frostline.
- d. All finish grade shall slope away from foundation for a minimum of 10'-0".
- e. An assumed soil bearing pressure is determined and will be increased in accordance with IBC Table 1806.2
- Fill material shall be free from debris, vegetation, and other foreign substances. g. Backfill trenches shall be compacted to 90% relative density per ASTM D1557 to within 12" of finished grade. The top 12" shall be landscape fill.
- h. Backfill at pipe trenches shall be compacted on both sides of pipe in 6" lifts.
- Waterproof exterior faces of all foundation walls adjacent to usable spaces. Waterproofing of all foundation and retaining walls to be the responsibility of the owner and/or contractor.
- All backfill against foundation walls must be compacted to 90% relative density, unless otherwise directed by a soils report. k. Perforated pipe sub-drain typical behind all retaining walls. Use 4" diameter PVC except where
- noted otherwise. Slope pipe to drain to daylight and drywell. **DIVISION 3 - CONCRETE:**
- a. All concrete shall have a minimum 28 day compressive strength of 3000 psi. To accommodate the "Severe Weather for Concrete" category, concrete shall have a minimum 28 day compressive strength of 3000 psi for foundation walls and other vertical concrete exposed to weather and a minimum compressive strength of 3500 psi for slabs, porches and other exterior flatwork, including garage slabs, exposed to weather as recommended by Table R402.2 of the IRC and Section 1904.1 of the IBC. No Special Inspection is required as design assumes 2500 psi.
- b. Reinforcement shall be per ASTM A615 grade 60 ksi, U.N.O. 2. Lap reinforcing Per Detail 5/S0.2, U.N.O.
- d. Reinforcement cover in cast-in-place concrete shall be as follows: (ACI Table 20.6.1.3.1) - 3" Concrete cast against and permanently exposed to earth. - 1 1/2" Concrete exposed to earth or weather with #5 bars or smaller.
- 0 3/4" Concrete not exposed to weather or in contact with ground, #11 bars and smaller, slabs, joists and walls,
- 1 1/2" Concrete not exposed to weather, beams, columns and pilaster, cover over ties. - 1 1/2" Clear to top for reinforcement in slabs on grade.
- e. All slabs on grade, S.O.G., shall have a minimum thickness of 4" and be reinforced with #3 at 18" o.c., or with Fibermesh as per manufacturers specifications equivalent to reinforcement specified above, U.N.O.
- Concrete shall be air-entrained to 6% +/- 1%. (For exterior slabs only) q. Provide slab control joints (saw cut or plastic inserts) at 10'-0" maximum spacing each way for 4" slab. Joint depth to be 1/4 of slab depth.
- h. All Post Tension concrete shall be Type II and have a minimum 28 day compressive strength of 4000 psi, U.N.O.
- Post Tension Concrete shall be air-entrained per Post Tension Designer.

#### THESE NOTES APPLY TO ALL SHEETS:

- It shall be the contractor's direct responsibility to comply with typical details and general notes as delineated or defined on the typical detail drawings of these contract documents regardless of specific flagging or reference to applicable note or detail.
- It shall be the contractor's responsibility to coordinate with all trades regarding utilities passing through and under footings. Structural requirements for these conditions are delineated in typ. details.
- Top of footing elevations noted are minimum. See note 2 for additional requirements. Contractor to verify and coordinate all locations and sizes of openings in slabs, slab depressions, and curbs for all related construction prior to floor layout or construction. Contractor shall then use appropriate details or appropriate wall section for each applicable
- condition Contractor to verify dimensions with architect prior to construction. Drawings are diagrammatic in nature and are not intended to indicate every opening or
- penetration in roof or other structure. Contractor shall coordinate and verify location and size of all such openings and penetrations with related subcontractors prior to roof or other framing layout or construction. Contractor shall then use appropriate typical or referenced details for each opening or penetration. Contractor to verify with appropriate sub-contractors the exact location, weight, and
- intended method of attachment of all items to be suspended from or in any way attached to any roof framing or other structural member unless such item(s) are clearly addressed by the structural construction documents. This information shall be transmitted in writing to structural engineer prior to final design or fabrication of structural framing members. Contractor to verify all existing conditions and dimensions and notify the architect in writing
- of any discrepancies.

#### SPECIAL INSPECTIONS AND DEFERRED SUBMITTALS:

- Special inspection, per the International Building Code chapter 17, AISC 360 and Table 1705.2.3 for steel and 1705.3 for concrete shall be required for the following types of work. See project Specifications for specified requirements:
- All concrete work for strengths greater than 2500 psi, except for slabs on grade, footings and non structural concrete. All reinforcing steel for concrete strengths greater than 2500 psi.
- All field welding (except metal studs, furring channels, etc.). Shop welding for procedures and multiple pass welds
- All full penetration welds shall be specially inspected in accordance with AWS and the current International Building Code. All fillet welds shall be visually inspected in accordance with AWS and the current
- International Building Code. All masonry work, see notes under `MASONRY' for requirements. All masonry inspection
- shall also comply with the National Concrete Masonry Institute. Bolts installed in conc. or masonry. Does not include sill PL, anchor bolts and Holdown anchor bolts.
- All ASTM A-325 and/or ASTM A-490 High Strength Bolts
- All expansion bolts and adhesive anchors.
- All grouted dowels. All insulating concrete.

DIVISION 4 - MASONRY:

- wall, U.N.O.
- **DIVISION 5 METALS:** a. All hardware called for shall be Simpson Strong-Tie Co, Inc. and installed per the manufacturer's
- specifications, U.N.O.
- weather proofing methods may be used.
- U.N.O.) the manufacturer.
- fabrication.

- standard cut washers.
- diameters from each end of the piece. 2308.3.1.1 for alternate.

#### **DIVISION 6 - WOOD:**

- covering, U.N.O.
- 303 (T1-11), or approved equal.
- all floors, U.N.O.
- and/or soil to stud wall contact.
- ground to be preservative treated Douglas Fir
- in accordance with ASTM A153. k. All framing members specified in these calculations and/or plans are minimums, and larger members of equal or better grade may be substituted. I. All floor openings shall be between joists, U.N.O. m. Do NOT notch beams, joists, and studs, U.N.O.
- n. Provide double joists below all parallel partition walls.
- o. No green lumber at time of covering shall be used on this project. No framing of any type shall be concealed prior to inspection by governing agencies. Sawn lumber shall have the following minimum grades (U.N.O.): - all 4x12 & smaller framing members - all 4x14, 4x16, 6x & 8x framing members 4x4 posts all other posts and timbers

- all 2x joists and rafters all 2x & 3x studs (unbra
- all 2x & 3x studs (unbra
- all 2x top plates ...... all 2x and 3x sills
- manuf. truss components . r. All resawn and roughsawn beams are to be free of heart center.
- to be 2". t. All multiple studs to be attached with 16d's at 12" o.c.

**ROOF SHEATHING FASTENING SCHEDULE ROOF FASTENING ZONE** PANEL ZONE 2 ZONE 3 ZONE 1 NAILS LOCATION FASTENER SCHEDULE (INCHES ON CENTER) SEE NOTE #2 PANEL EDGES 6 6 4 SEE NOTE #2 PANEL FIELD 12 6 6

1. BLOCKING NOT REQUIRED, UNLESS NOTED OTHERWISE ON PLANS. 2. USE 8d NAILS FOR 1/2" ROOF PLY & 10d NAILS FOR 5/8" ROOF PLY.

## 4' TYP



#### **HIP ROOF**

#### **ROOF SHEATHING:**

(E) 5/8" CDX APA Rated (32/16) or OSB equivalent, Apply face grain perpendicular to framing. Stagger panels and nail with 8d's common at 6" o.c. at edges and boundaries (blocking, drag trusses, shear blocks, etc.), and 12" o.c. field. U.N.O. (See special diaphragm nailing requirements this sheet)

#### **FRAMING MATERIAL:**

<u>ROOF HEADERS</u> 6x8 D.F. #1 (4x8 D.F. #2 at 2x4 walls) or RMT U.N.O. Provide (2) Trimmers at openings greater than 5'-0" U.N.O.

WALL FRAMING Use 2x6 or 2x4 D.F. Stud or Construction Grade at 16" o.c. as occurs Typ\_U.N.O. FLOOR PLYWOOD 3/4" T & G APA rated plywood (or OSB). Apply face grain perpendicular to framing. Stagger panels and nail with 10d's at 6" o.c. at all edges and boundaries (blocking at interior shear walls, drag members,

- etc.), and 10" o.c. field. FLOOR FRAMING

c. Vertical steel placement in masonry walls to be #4 bars at 16" o.c. maximum spacing centered in

Horizontal steel placement in masonry walls to be #4 bars at 24" o.c. maximum spacing, U.N.O.

b. Structural steel shall conform to ASTM A992, grade 50 U.N.O. Miscellaneous steel such as plates, channels and angles may be ASTM A36. Steel pipe columns shall conform to ASTM A53, Type E or S. Steel tube sections shall conform to ASTM A500, Grade B.

c. All steel exposed to weather shall be hot-dip galvanized after fabrication or other approved Where finish is attached to steel provide 1/2" dia. bolt holes at 36" o.c., U.N.O.. For attachment of nailers see architectural drawings for finishes. (alternate 1/2" dia. x 3" nelson studs at 36" o.c.,

e. All grout under steel bearing plates shall be solid drypack or non-shrink grout placed as directed by

f. Shop drawings shall be submitted to the Structural Engineer for review and comment prior to

g. All welding shall conform to the American Welding Society specifications. All welding shall be performed by certified welders approved by the local building authority. All shop welding shall be in an approved fabricator's shop authorized by the local building authority or special inspection per the IBC shall be provided. All field welding shall require special inspection per IBC Section 1704. All welding electrodes shall be E70XX or shielded wires with Fy = 70 ksi.

All nails specified are common nails. No substitutions unless approved in writing by Dunagan Engineering, Inc. or specifically addressed in these calculations or the plans. All nails exposed to weather shall be galvanized. Fasteners for pressure-preservative treated and fire-retardant treated wood shall be of hot-dipped zinc coated galvanized, stainless steel, silicon bronze or copper. The minimum nailing for all framing shall conform to IBC Table 2304.10.1.

All bolts specified must meet ASTM A307. Bolt holes to be 1/32" to 1/16" larger than specified bolt. Washers shall be used at each bolt head and nut next to wood. All washers to be not less than

Wood plates or sills shall be bolted to the foundation or foundation wall. Steel bolts with a minimum nominal diameter of 1/2" shall be used. Bolts shall be embedded at least 7 inches into the concrete or masonry. In a two pour system embedment shall be into the first pour. There shall be a minimum of two bolts per piece with one bolt located not more than 12 inches or less than 7 bolt

m. Plate washers a minimum of 3"x3"x1/4" thick shall be used on each bolt. See IBC section

a. All lumber framing shall be Douglas Fir Larch (DOC PS20) with moisture content < 19% at time of

b. Glu-Lams used for simple spans shall be 24F-V4 U.N.O. Glu-Lams used for continuous spans or cantilever shall be 24F-V8, U.N.O. Glu-Lams exposed to weather shall be rated for exterior use by manufacturer or approved protection from exposure to be provided.

c. All plywood shall conform to APA DOC PS1 or DOC PS2. All shear plywood shall be C-D, C-C, Where multiple trimmers or studs are specified, those trimmers are to be stacked in all wall framing and solid blocking to be provided at all floors down to the foundation.

e. Where posts with column caps, straps, or bearing plates are called for, the load is to be transferred to the foundation with posts as specified in the plans and solid vertical grain blocking at

All studs to be stud grade or better, U.N.O. In no instance shall a stud wall be used to resist lateral pressures due to snow or soil. It is the owner and/or contractor's responsibility to eliminate snow

g. All laminated veneer lumber (LVL) and parallel strand lumber (PSL) specified shall have the following minimum design strengths: 1 3/4" wide: Fb=2600 psi, Fv=285 psi, E=1,900,000 psi. 3 1/2" wide and greater: Fb=2900 psi, Fv=290 psi, E=2,000,000 psi. h. All multiple-ply LVL members to be attached with (3) rows of 16d common nails at 12" o.c. for

entire length of member. For a three piece member the nailing is from each side. Foundation sill plates, nailers, and ledgers in direct contact with concrete and within 6 1/2" of

Fasteners for preservative treated and fire treated wood shall be of hot dipped, zinc coated, galvanized steel, silicon, bronze or copper. The coating weights for zinc coated fasteners shall be

#2	
stud or construction	aced length up to 10')
standaro	
standaro	

s. Double joists shall be attached with (2) rows of 16d's at 12" o.c. edge distance of nailing

..grade per manuf.

**GABLE ROOF** 

Use 4x6 P.T. D.F. #2 16" o.c. or approved equal installed per manufacturer's specifications, Typ. U.N.O. 

#### **DESIGN CRITERIA** 2018 International Building Code (IBC) Local Building Department Standards Soil Bearing (IBC Table 1806.2)

#### WIND DESIGN DATA

Ultimate Design Wind Speed, Vu = 120 m.p.h. (3-Second Gust) Risk Category II Wind Importance Factor, Iw = 1.00

Wind Exposure C Internal Pressure Coefficient = +/- 0.18

Components & Cladding Design Pressures (ASCE 7 Section 30.4.2): a = 6.8 ft (ASCE 7 Figure 30.4-1)

Refer to ASCE 7-16 Figure 30.4-1 for layout.				
Boof/Wall	7000	Effective Wind Area	Design Wind Pressure, P <sub>net</sub>	
R001/Wall	Zone	(ft <sup>2</sup> )	(psf)	
	1	10	44.6	
	1	20	44.6	
	1	50	37.9	
Å.	1	100	32.8	
02	2	10	71.1	
0 t	2	20	62.3	
~ 2	2	50	50.7	
of	2	100	41.9	
Ř	3	10	91.7	
	3	20	74.9	
	3	50	52.6	
	3	100	52.6	
	4	10	34.0	
	4	20	32.5	
	4	50	30.7	
冒	4	100	29.3	
Ň	5	10	42.0	
	5	20	39.2	
	5	50	35.5	
	5	100	32.5	

SEISMIC DESIGN DATA

Importance Factor, le = 1.00 (Risk Category II) Ss = 1.945 g and S1 = 0.686 g

Site class: = D SDs = 1.297 g , SD1 = 0.777 g

Seismic design category: = D

Basic seismic-force-resisting system(s): = Light-Framed Walls Sheathed with Wood Structural Panels Rated

for Shear Resistance, R = 6.5 N/S Design Base Shear (LRFD) = 14.9 kips

E/W Design Base Shear (LRFD) = 14.9 kips

Cs (LRFD)= 0.1994 Analysis Procedure Used = Equivalent Lateral Force Procedure

**SNOW LOAD DATA:** Site Elevation

Ground Snow Load	Pg =	30 psf
Flat-Roof Snow Load	Pf =	21 psf
Snow Exposure Factor	Ce =	0.9
Snow Importance Factor	ls =	1.0
Thermal Factor	Ct =	1.1

< 5000 FT.

FLOOR FRAMING DESIGN LOADS		LOWER
Floor Live Load =		40 PSF
Floor Dead Load =		S.O.G.
Total Floor Load =		40 PSF
<b>CEILING JOIST LOADING:</b>	TYPICAL	
Storage Load =	20 PSF	
Dead Load =	5 PSF	
Total Load =	25 PSF	
(E) PURLIN LOADING:	TYPICAL	
Snow Lood =	21 DSE	

now Load =	21	PS
)ead Load =	15	PS
otal Load =	36	PS

CONNECTIO	N CROSS R	EFERENCE					
impson Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors	Simpson Strong-Tie	USP Structural Connectors
Product Number	Product Number	Product Number	Product Number	Product Number	Product Number	Product Number	Product Number
SSTB16	STB16	CB66	KCB66	HU410	HD410	HGUS26-3	THDH26-3
SSTB24	STB24	CB68	KCB68	HU412	HD412	HGUS28-3	THDH28-3
HDU5-SDS2.5	PHD5	HUCQ1.81/9-SDS	HDQ179IF	HU68	HD68	TJC37	SNP3
HDQ8-SDS3	UPHD8	HUCQ1.81/11-SDS	HDQ17112IF	HU610	HD610	THJA26	HJC26
HDQ11-SDS2.5	UPHD11		HDQ1714IF	HU612	HD612	MTHM	HJHC26
HDQ14-SDS2.5	UPHD14	HUCQ310-SDS	HDQ310IF	LSU26	LSSH15-TZ	DSC4R/L-SDS3	DSC4R/L
HD15	TD15	HUCQ210-2-SDS	HDQ210-2IF	LSSU28		ST6224	KST224
ABU44	PAU44	HUCQ410-SDS	HDQ410IF	LSSU210	LSSH210	CS16	RS150
ABU46	PAU46	HUCQ412-SDS	HDQ412IF	SUR/L24	SKH24R/L	MSTC48B3	
ABU66	PAU66	HUCQ210-3-SDS	HDQ210-3IF	SUR/L26	SKH26R/L	H1	RT15
ABU88	PAU88	HUCQ5.25/9-SDS	HDQ5210IF	SUR/L210	SKH210R/L	H2.5A	RT7A
PB44	WE44	HUCQ5.25/11-SDS	HDQ5212IF	IUS	THF	H2A	RT10
PB46	WE46	HUCCQ610-SDS	HDQ610IF	HU11	HD17112	HGA10KT	HGA10
PB66	WE66	HUCQ612-SDS	HDQ612IF	IUT	THF	A34	MP34
CBQ44	KCBQ44	LUS24	JUS24	ITS	THO/TFL	A35	MPA1
CBQ46	KCBQ46	LUS26	JUS26	ITT	THO/TFL	LTP4	MP4F
CBQ66	KCBQ66	LUS28	JUS28	LUS26-2	JUS26-2	LS50	MP5
CB44	KCB44	LUS210	JUS210	HHUS26-2	THD26-2	LS70	MP7
CB46	KCB46	LUS46	JUS46	HGUS26-2	THDH26-2	LS90	MP9
CB48	KCB48	HU46	HD46	HHUS28-2	THD28-2	CCQ/ECCQ	KCCQ/KECCQ

#### HOLDOWN SPECIFICATION TABLE

		(ALSO S	SEE SIMPSON STRONG	G-TIE CATAI	LOG)			
		POST		Т	HREADED ROD		SSTB BC	DLT
		MIN.	SCREWS, BOLTS		EMBEDMENT			
HOLDOWN	CL	THICKNESS	OR NAILS	A.B. DIA.	8" STEM WALL	FOOTING	SGL. POUR	DBL. POUR
HTT4	1 5/16"	3"	(18) 16d's x 2 1/2"	5/8"	18"	-	SSTB24	SSTB24
HTT5	1 5/16"	3"	(26) 16d's x 2 1/2"	5/8"	24"	-	SSTB28	SSTB28
HDU5	1 5/16"	3"	(14) SDS 1/4"x2 1/2"	5/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	SSTB28	THRD. ROD OPTION ONLY
HDU8	1 3/8"	4 1/2"	(20) SDS 1/4"x2 1/2"	7/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HDQ8	1 1/4"	4 1/2"	(20) SDS 1/4"x3"	7/8"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HHDQ11	1 1/2"	5 1/2"	(24) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HHDQ14	1 1/2"	5 1/2"	(30) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HDU14	1 9/16"	5 1/2"	(36) SDS 1/4"x2 1/2"	1"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A
HD19	2 1/8"	5 1/2"x5 1/2"	(5) 1" DIA. BOLTS	1 1/4"	SEE HOLDOWN SCHEDULE PER PLAN	SEE HOLDOWN SCHEDULE PER PLAN	N/A	N/A

NAIL SPECIFICATIONS NOMINAL NOMINAL MIN. EMBED FOR MIN. NAIL LENGTH NAIL TYPE DIAMETER (GAGE) LENGTH P.W. SHEATHING PLY. THICKNESS 0.113" (11 ga.) 6d COMMON 2" 1 3/8" 0.131" (10 1/4 ga.) 8d COMMON 2 1/2" 1 3/8" \_\_\_\_\_ 0.148" (9 ga.) 3" 1 3/4" 10d COMMON 3 1/2" 12d COMMON 0.148" (9 ga.) 0.162" (8 ga.) 3 1/4" 16d COMMON -MIN. EMBED. 16d G.V. SINKER 0.148" (9 ga.) 3 1/4" -DETERMINE REQ'D NAIL DIAMETER AND LENGTH REQUIRED COMMON NAIL 

PLYWOOD THICKNESS	3/8"	1/2"	5/8"	3/4"	1 1/8"	3/8"	1/2"	5/8"	3/4"	1 1/8"
MINIMUM EMBEDMENT		1 3/8"				1 3/4"				
MIN. NAIL LENGTH REQ'D	2"	2 1/8"	2 1/4"	2 3/8"	2 3/4"	2 1/8"	2 1/4"	2 3/8"	2 1/2"	2 7/8"
MIN. DIAMETER REQ'D		0.131'	' (10 1/4"	ga.)			0.148	" (10 1/4"	ga.)	

HREADED ROD END

ONDITION AT HOLDOWNS

See holdown specification

(2) Nuts & washer as shown

table on this sheet for

threaded rod size.

#### FOOTING AND STEMWALL REQUIREMENTS

Wide w/ (1) #4 continuous top and #4 at 48" o.c. vertical, hook at footing (alternate hooks). Locate vertical at all Holdown Anchor Bolts. If top of stemwall exceeds 36" above top of footing, use #4 at 18" o.c. horizontal continuous and #4 at 24" o.c. vertical.

· All footings shall bear on undisturbed soil. Assumed soil bearing pressure

is determined & increased in accordance w/ IBC Table 1806.2. • Exterior footings to be placed 24" below grade minimum, U.N.O.

HOLDOWN INFORMATION • See holdown schedule above and per plan.

#### SOILS & FOUNDATIONS:

Dunagan Engineering, Inc. has not made a geotechnical review of the building site and is not responsible for general site stability or soil suitability for the proposed project. A review by a geological engineer or qualified civil engineer may be desirable. Foundation design is based on minimum footing dimensions and bearing capacities set forth in Table 1806.2 of Chapter 18 in the 2018 International Building Code. Assume Class 5 soil with allowable soil bearing pressure of 1500 psf., with a constant expansion index less than 20. Footings shall extend 24" (minimum) below grade.





REVISIONS

05-20-21

Description PLAN CHECK



DRAWN BY	1
KML	
CHECKED BY	
EHS	
DATE	
4-23-21	
SCALE	
AS NOTED	
JOB NO.	
BB20121	
SHEET NO.	
	1
GENERAL NOTES & TYPICAL DETAILS	
S0.1	
SHEET of SHEETS	















### 16"x8" FLOOD VENTS FOR GARAGE - (1) ON EACH SIDE OF BUILDING, (2) TOTAL $\sqrt{2}$

\_\_\_\_\_

\_\_\_\_

#### FLOOR FRAMING NOTES

<u>SUBFLOOR:</u> 3/4" PLYWOOD SHEATHING, EXPOSURE 1, T & G UNDERLAYMENT GRADE, APA SPAN RATED 24" o.c. or EQUIVALENT, LAID AT RIGHT ANGLES OVER FLOOR JOISTS. STAGGER JOINTS, GLUE & NAIL WITH 10d's AT 6" o.c. EDGE, 10" o.c. FIELD.

FLOOR JOISTS: 4x6 PT DF #2 OR SINGLE SPAN 2x8 PT DF #2 FLOOR JOISTS AT 16" o.c. w/ LUS46 HANGERS U.N.O., INSTALLED PER MANUFACTURER'S SPECIFICATIONS AT RIGHT ANGLES OVER BEARING. DOUBLE UNDER PARALLEL WALLS. BLOCK ALL JOISTS AT BEARING POINTS PER MFR. SPECIFICATIONS.

GIRDERS: 6x8 PT DF #1 GIRDERS, U.N.O.

<u>RIM:</u> 1 1/4" TIMBERSTRAND LSL U.N.O.

<u>SILLS & PADS:</u> 2x PRESSURE TREATED LUMBER (TYP.), or TIMBERSTRAND LSL TREATED SILL PLATES PER ICC-ES ESR 1387.

NOTE: SEE STRUCTURAL FLOOR PLANS FOR LOCATION OF HOLDOWNS. USE RIM BLK'G OR DBL. SOLID BLK'G AT HOLDOWN HTT4, HTT5, HDU5, HDU8, AND HDQ8. USE VERTICAL GRAIN SOLID BLK'G. TO MATCH HOLDOWN STUD AT HOLDOWN HHDQ11,HHDQ14, AND HDU14, AND HD19.

# CONVERSION 1210 MILE CR. RENO, NV 89511 APN: 043-062-18 RAGE GA S **GREVE**

### SUBMITTAL SET



/1\



#### **ROOF FRAMING NOTES**

<u>SHEATHING:</u> (E) 5/8" CDX PLYWOOD (or EQUAL) EXPOSURE 1, APA SPAN RATED (32/16). STAGGER JOINTS, NAIL w/ 8d AT 6" o.c ALL EDGES, GABLE ENDS AND FRIEZE BLOCKS. NAIL w/ 8d AT 12" o.c. FIELD. ALL PLYWOOD SHALL CONFORM TO APA PS 1. ALL SHEAR PLYWOOD SHALL BE C-D, C-C, 303 (T1-11), or APPROVED EQUAL.

- LVL's, PSL's & LSL's: ALL LVL's SHALL HAVE Fb= 2600 PSI, Fv= 285 PSI, AND E=2.0x10^6 PSI MIN.
- UNLESS NOTED OTHERWISE NAIL MULTI-PLY LVL's w/ (3) 16d's AT 12" o.c. ALL PSL's SHALL HAVE Fb= 2900 PSI, Fv= 290 PSI, AND E=2.0x10^6 PSI MIN. U.N.O.
- ALL LSL'S SHALL HAVE Fb= 2250 PSI, Fv= 400 PSI, AND E=1.5x10<sup>6</sup> PSI MIN. UNLESS NOTED OTHERWISE NAIL MULTI-PLY LVL'S w/ (3) 16d'S AT 12" o.c.

HEADERS: 6x8 ROSBORO MFG. TIMBER or DF #1 TYP., U.N.O. 4x8 ROSBORO MFG. TIMBER or DF #2 AT 2x4 WALLS TYP., U.N.O.

<u>TRIMMERS:</u> DBL. TRIMMERS AT OPENINGS GREATER THAN 5'-0", AT 2x6 WALLS TYP. U.N.O. DBL. TRIMMERS AT OPENINGS GREATER THAN 4'-0", AT 2x4 WALLS TYP. U.N.O.

<u>POSTS:</u> 4x D.F. #2 AND 6x D.F. #1 (LOCATE AS NOTED)

METAL CONNECTORS: (USE SIMPSON BRAND or APPROVED EQUAL). HANGERS SHOWN AT TRUSSES ARE TYPICAL, PROVIDE HANGERS AS SPECIFIED ON THE STAMPED TRUSS CALCULATIONS. SIMPSON H1 CLIPS AT ALL TRUSS BEARING POINTS ON PLATES & BEAMS

SIMPSON H5 CLIPS AT ALL RAFTER BEARING POINTS ON PLATES & BEAMS SIMPSON H2.5A CLIPS (B/S) AT ALL GIRDER TRUSS BEARING POINTS. SIMPSON POST CAPS (AS NOTED)

SIMPSON ST6224 (AS NOTED)



## CONVERSION E CR. / 89511 -062-18 RAGE MILI , NV 043-( 210 N NO, GA 12 REI API GREVES

CHECKED BY EHS DATE 4-23-21 SCALE AS NOTED JOB NO. BB20121 SHEET NO. CEILING FRAMING PLAN AND ROOF FRAMING PLAN AND ROOF FRAMING PLAN SHEET of SHEETS	DRAWN BY KML
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SHEET NO. CEILING FRAMING PLAN AND ROOF FRAMING PLAN SHEET of SHEETS	JOB NO. BB20121
CEILING FRAMING PLAN AND ROOF FRAMING PLAN SHEET of SHEETS	SHEET NO.
S12.1 SHEET of SHEETS	CEILING FRAMING PLAN AND ROOF FRAMING PLAN
S1 SHEET of SHEETS	
SHEET of SHEETS	S2.1
	SHEET of SHEETS